Effects of Seismic Motion Incoherency on SSI and SSSI Responses of Nuclear Structures for Different Soil Site Conditions

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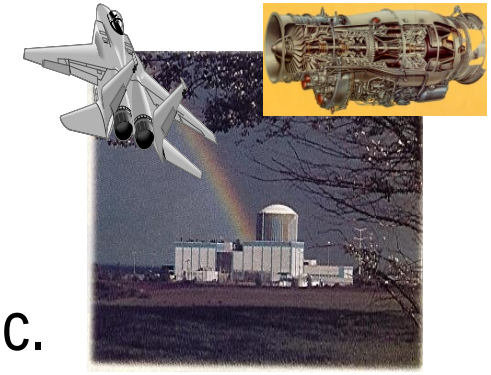

2014 DOE Natural Phenomena Hazards Meeting
Germantown, MD, October 21-22, 2014

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Purpose of This Presentation:

To disseminate results of some internal multiyear research projects done in GP Technologies for a better understanding of the incoherency effects onnuclear structure SSI and SSSI responses.

To answer to the following important questions:
- What is the meaning of “incoherent motion”?
- How the foundation flexibility impact on the incoherent SSI methodology and in-structure responses?
- How much can the foundation size influence incoherent responses?
- How much can the seismic input directionality affect incoherent results?
- How much can incoherency influence SSSI effects for rock and soil sites?

Discuss effects on ISRS, soil pressures, structural shear forces, and foundation wall bending moments, SSI relative displacement between neighboring buildings….

The new *ACS SASSI Version 3.0 code* was used for all these studies.

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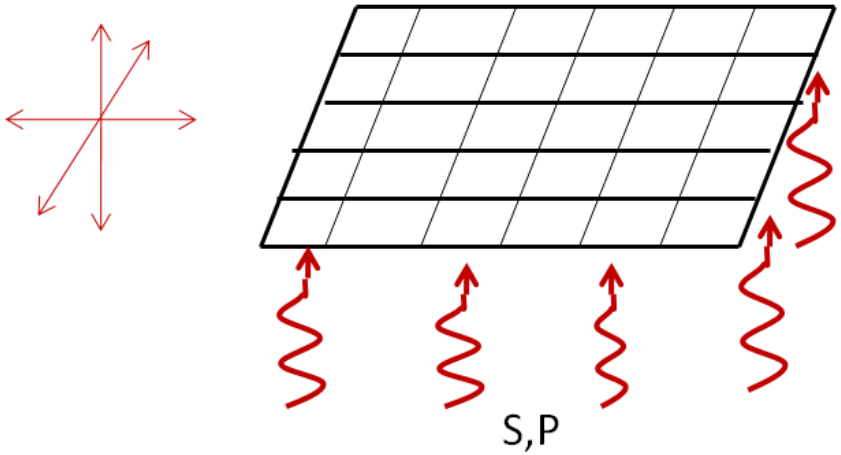
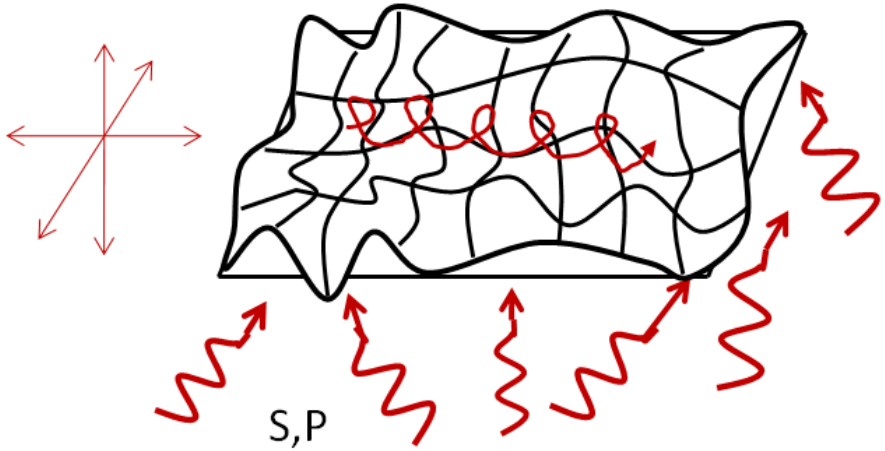
Coherent vs. Incoherent Wave Propagation Models

3D Rigid Body Soil Motion (Idealized)



3D Random Wave Field Soil Motion (Realistic)



1 D Wave Propagation Analytical Model (Coherent)

Vertically Propagating S and P waves (1D)
- No other waves types included
- No heterogeneity random orientation and arrivals included
- Results in a rigid body soil motion, even for large-size foundations

3D Wave Propagation Data-Based Model (Incoherent – Database-Driven Adjusted Coherent)

Includes real field records information, including implicitly motion field heterogeneity, random arrivals of different wave types under random incident angles.

ANIMATIONS

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Motion Incoherency Simulation in ACS SASSI

The complex frequency response is computed as follows:

- Coherent SSI response:

Structural transfer function given input at interaction nodes

Coherent ground transfer function at interface nodes given control motion

$$U_s(\omega) = H_s(\omega) * H_g^c(\omega) * U_{g,0}(\omega)$$

Complex Fourier transform of control motion

- Incoherent SSI response:

Incoherent ground transfer function given coherent ground motion and coherency model (random spatial variation in horizontal plane)

$$U_s(\omega) = H_s(\omega) * S_g^i(\omega) * H_g^c(\omega) * U_{g,0}(\omega)$$

$$S_g(\omega) = [\Phi(\omega)][\lambda(\omega)]\{\eta_\theta\}$$

Complex Fourier transform of relative spatial variations of soil motion at interaction nodes = stochastic wave field

Eigenmodes of coherency kernel (deterministic part)

Random phases (stochastic part)

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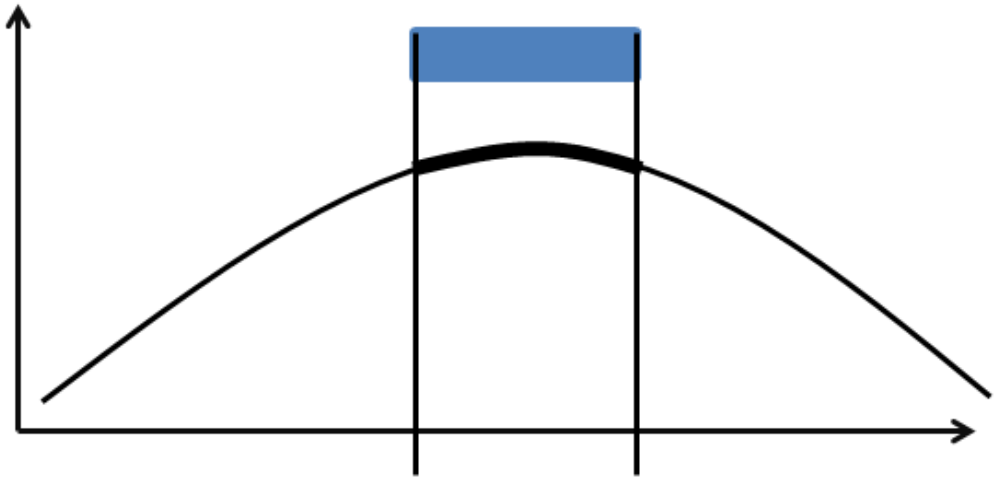
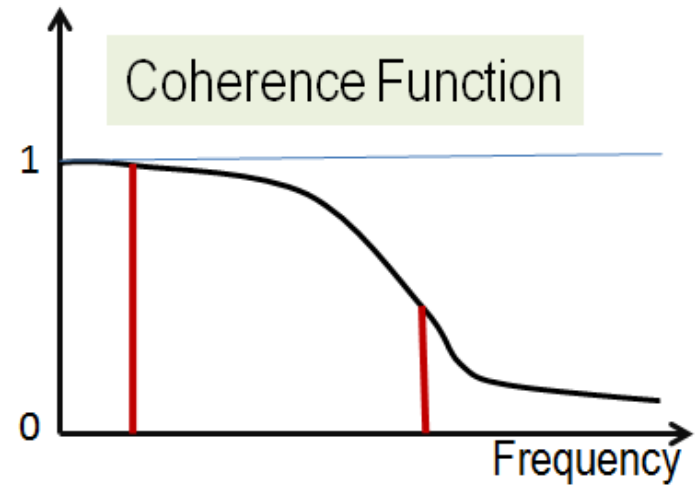
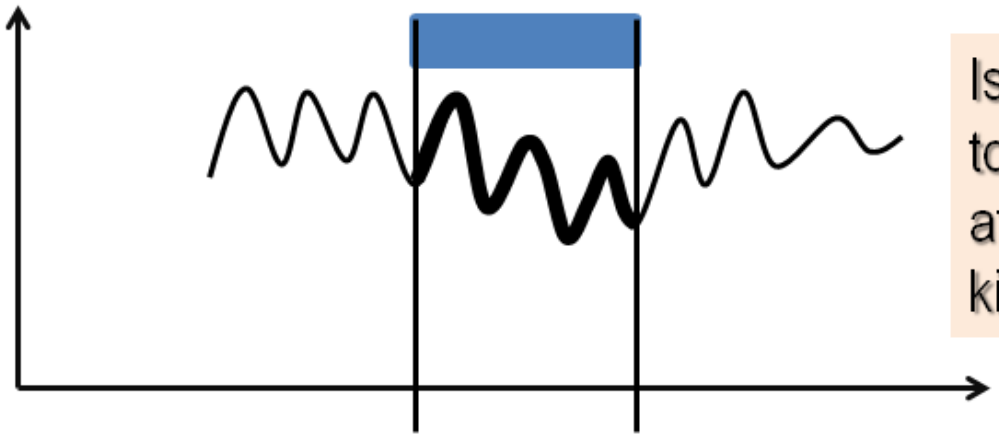
How Many Modes Do we Need to Consider ?

Low Frequency/Large Wavelengths/Only Few Low Order Incoherency Modes





High Frequency/Short Wavelengths/Low and High Order Incoherency Modes



Is the foundation sufficiently rigid to neglect high order modes at high frequency due to kinematic interaction effects?

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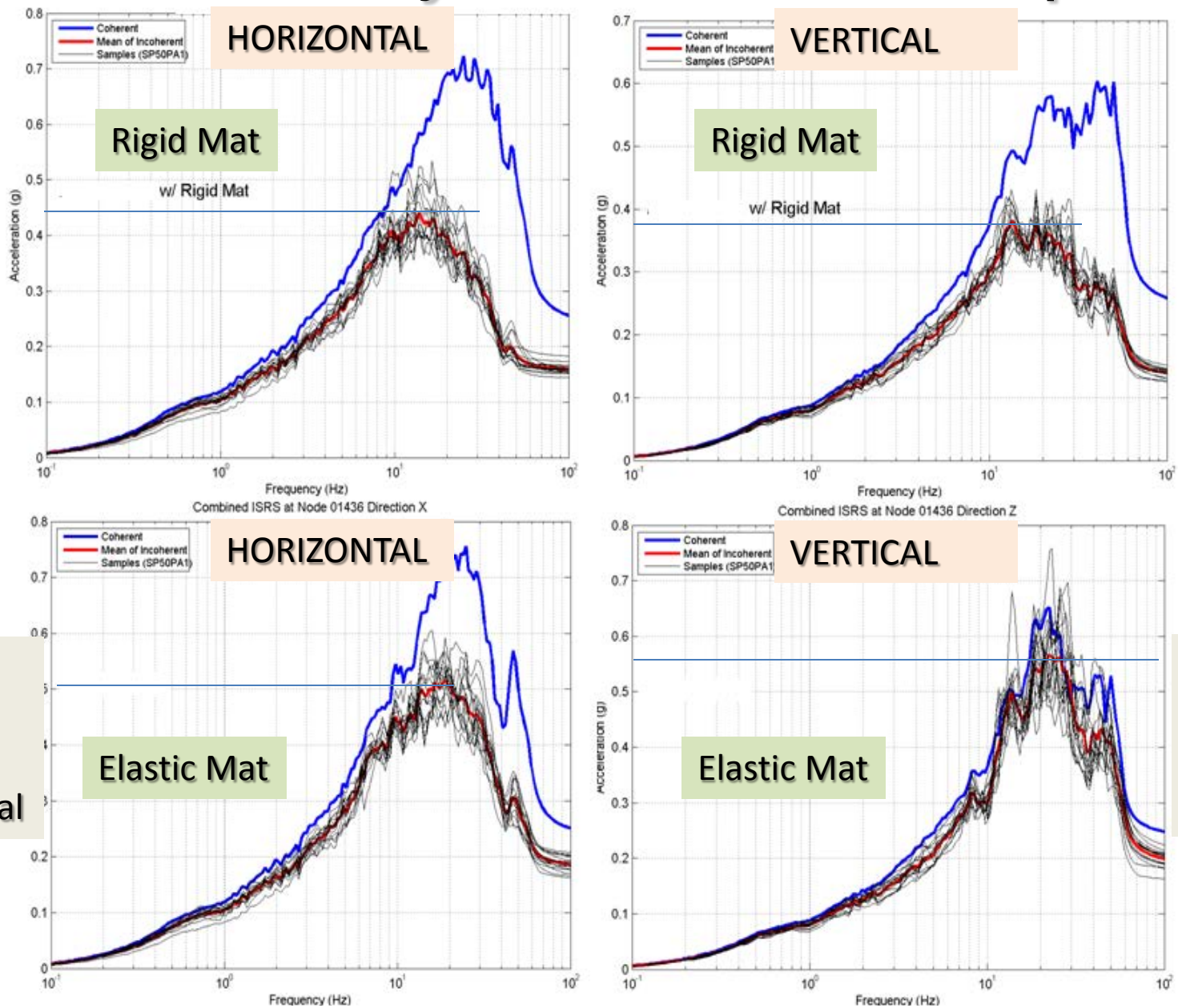
Basemat Flexibility Effects on RB Complex ISRS



Elastic is 20% up for horizontal

Elastic is 65% (!) up for vertical

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Cumulative Modal Contribution for 10 Modes

2007 Abrahamson Rock Site Model

*** CUMULATIVE MODAL MASS/VARIANCE(%) ***

Frequency =	0.098	Horizontal =	100.00%	Vertical =	100.00%
Frequency =	1.562	Horizontal =	100.00%	Vertical =	99.97%
Frequency =	3.125	Horizontal =	99.94%	Vertical =	99.75%
Frequency =	4.688	Horizontal =	99.69%	Vertical =	99.20%
Frequency =	6.250	Horizontal =	98.90%	Vertical =	98.09%
Frequency =	7.812	Horizontal =	97.01%	Vertical =	96.00%
Frequency =	9.375	Horizontal =	93.55%	Vertical =	92.59%
Frequency =	10.938	Horizontal =	88.54%	Vertical =	87.93%
Frequency =	12.500	Horizontal =	82.47%	Vertical =	82.46%
Frequency =	14.062	Horizontal =	75.90%	Vertical =	76.67%
Frequency =	15.625	Horizontal =	69.31%	Vertical =	70.92%
Frequency =	17.188	Horizontal =	63.02%	Vertical =	65.45%
Frequency =	18.750	Horizontal =	57.20%	Vertical =	60.37%
Frequency =	20.312	Horizontal =	51.92%	Vertical =	55.74%
Frequency =	21.875	Horizontal =	47.19%	Vertical =	51.55%
Frequency =	23.438	Horizontal =	42.99%	Vertical =	47.79%
Frequency =	25.000	Horizontal =	39.26%	Vertical =	44.40%
Frequency =	26.562	Horizontal =	35.96%	Vertical =	41.37%
Frequency =	28.125	Horizontal =	33.04%	Vertical =	38.65%
Frequency =	29.688	Horizontal =	30.42%	Vertical =	36.20%
Frequency =	31.250	Horizontal =	28.04%	Vertical =	34.00%
Frequency =	32.812	Horizontal =	25.81%	Vertical =	32.01%
Frequency =	34.375	Horizontal =	23.63%	Vertical =	30.21%
Frequency =	35.938	Horizontal =	21.37%	Vertical =	28.57%
Frequency =	37.500	Horizontal =	18.93%	Vertical =	27.09%
Frequency =	39.062	Horizontal =	16.31%	Vertical =	25.74%

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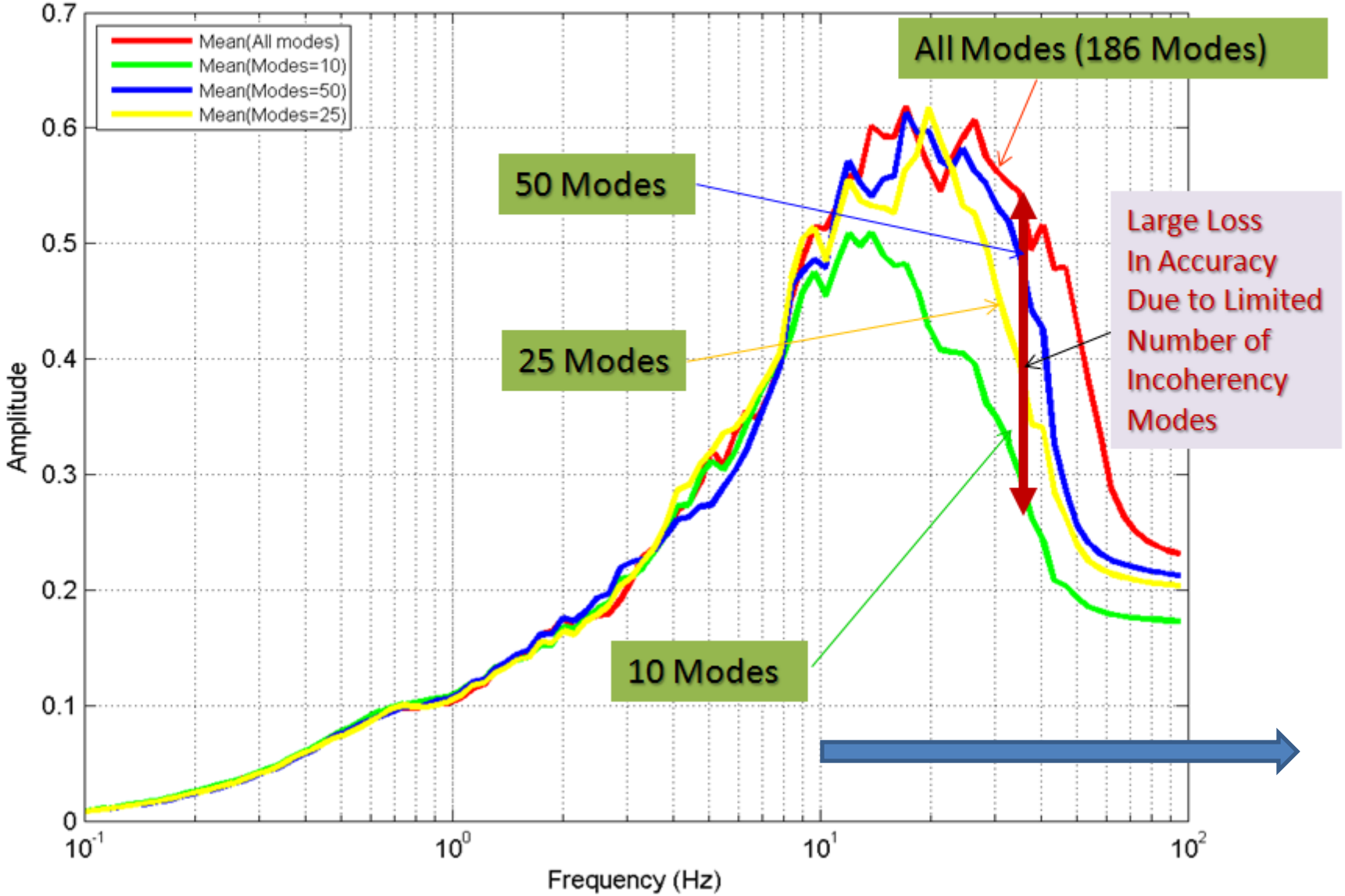
Effects of Number of Incoherent Modes on ISRS

Elastic Basemat Corner – X Direction



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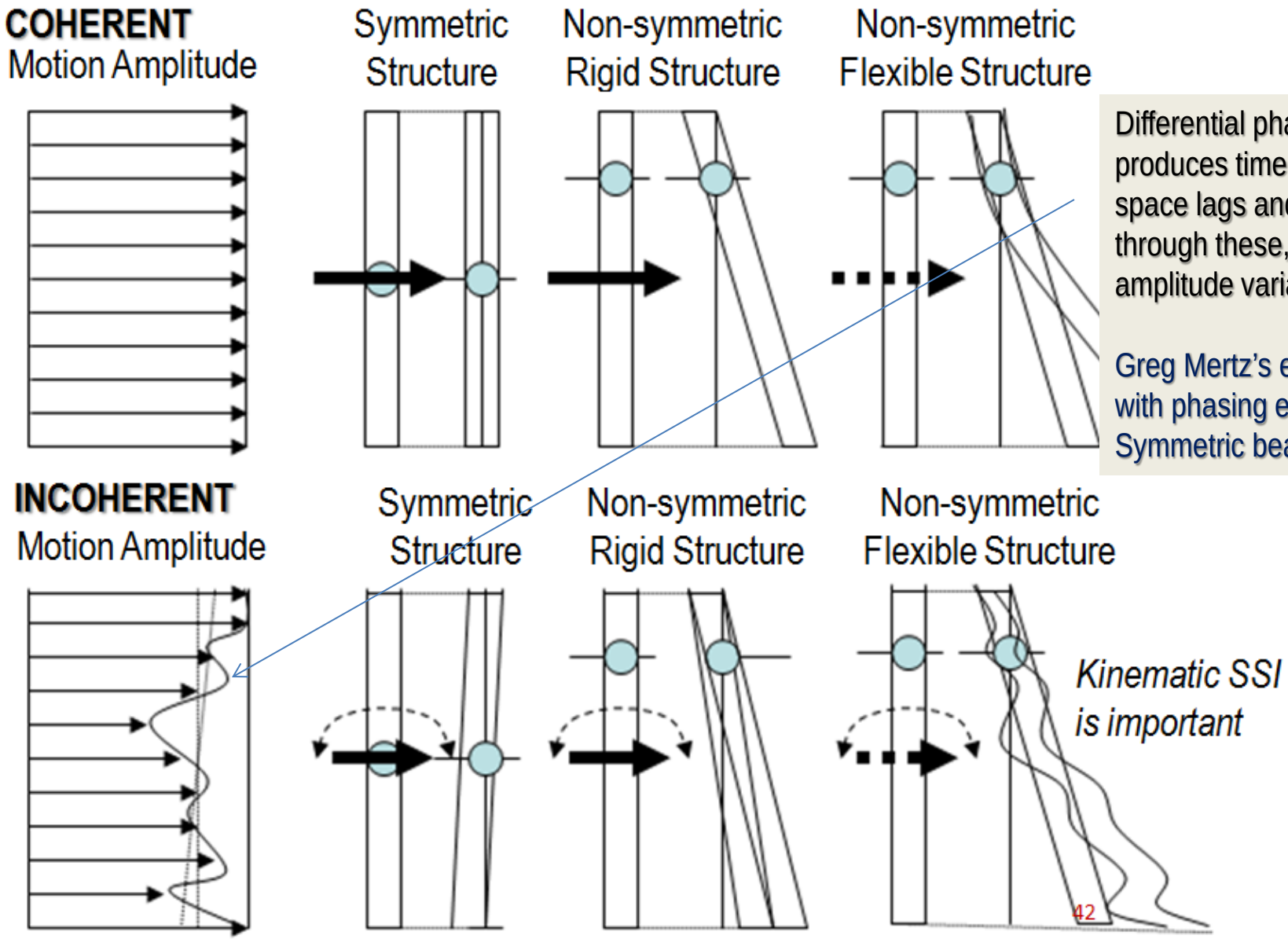
Motion Incoherency Differential Phasing Effects



Differential phasing produces time and space lags and through these, amplitude variations

Greg Mertz's example with phasing effect on Symmetric beam

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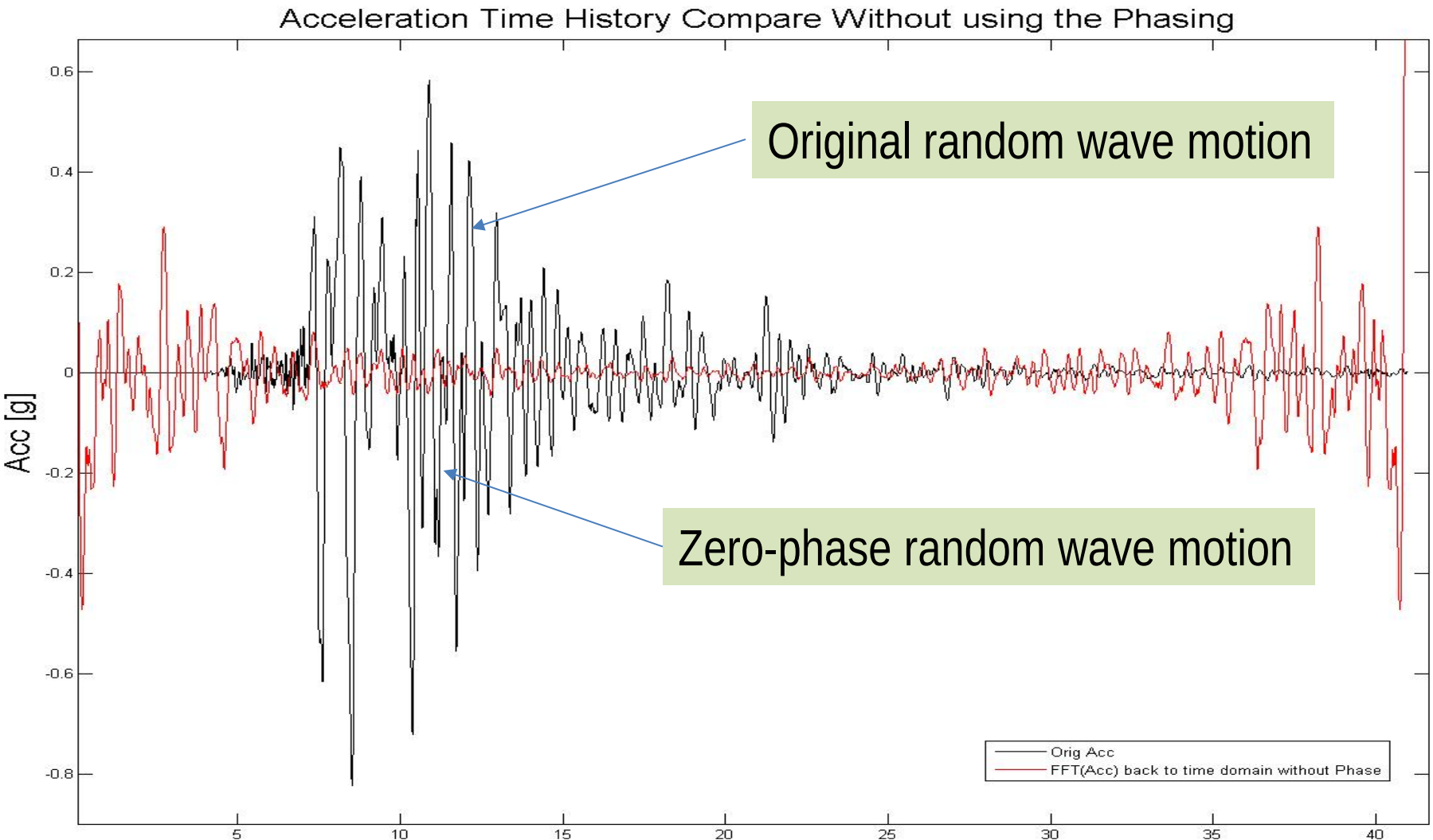
Effects of Zeroing Phases of Complex Responses in Frequency on Time Domain Responses

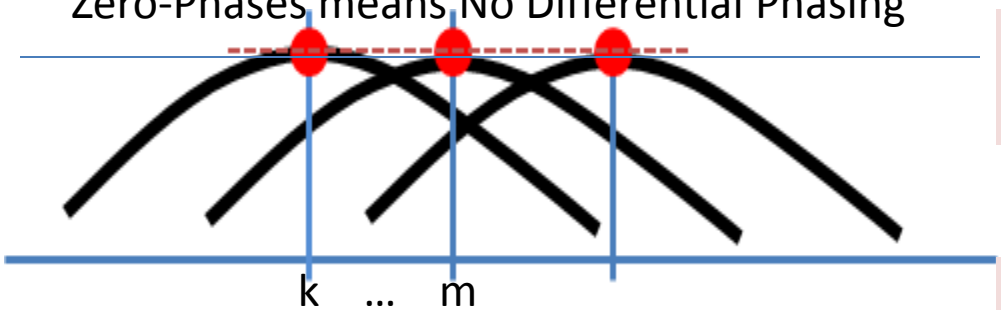
Fourier Transform Example



Effect of Zeroing Differential Phases at Lower-Mid Frequencies

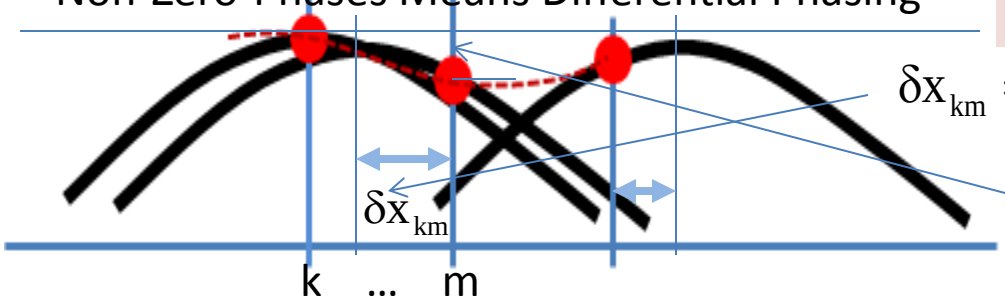
For dominant single mode situations (in lower frequency range), the *neglect of the (differential) phases* that produce random amplitude variations in space, *basically changes the problem and departs from reality.*

Zero-Phases means No Differential Phasing



Single Mode "Zero-Phase" Motion produces a "deterministic rigid body" motion

Non-Zero-Phases Means Differential Phasing



Single Mode "Non-Zero-Phase" Motion produces a "random field" motion

$$\delta x_{km} = (\psi_{j,k} - \psi_{j,m}) / k_j \quad \text{(space lag)}$$

Differential Amplitude Variations due to Differential Random Phasing

Mode 1 Contribution		
Freq	Part H	Part V
1 Hz	100%	98.2
8 Hz	84%	67%
25 Hz	7%	21%

At the lower frequencies, below 10 Hz, where a single mode (Mode 1) is governing, the zero-phase assumption practically neglects the differential phase variations between motion components due to incoherency.

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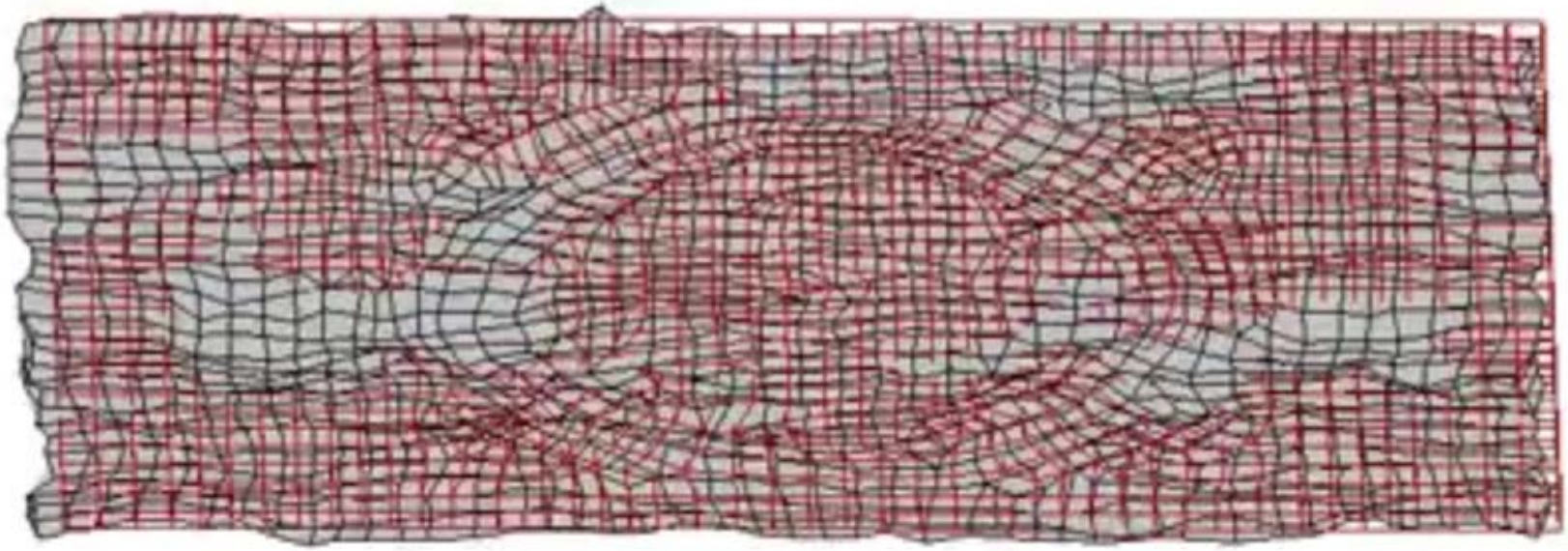
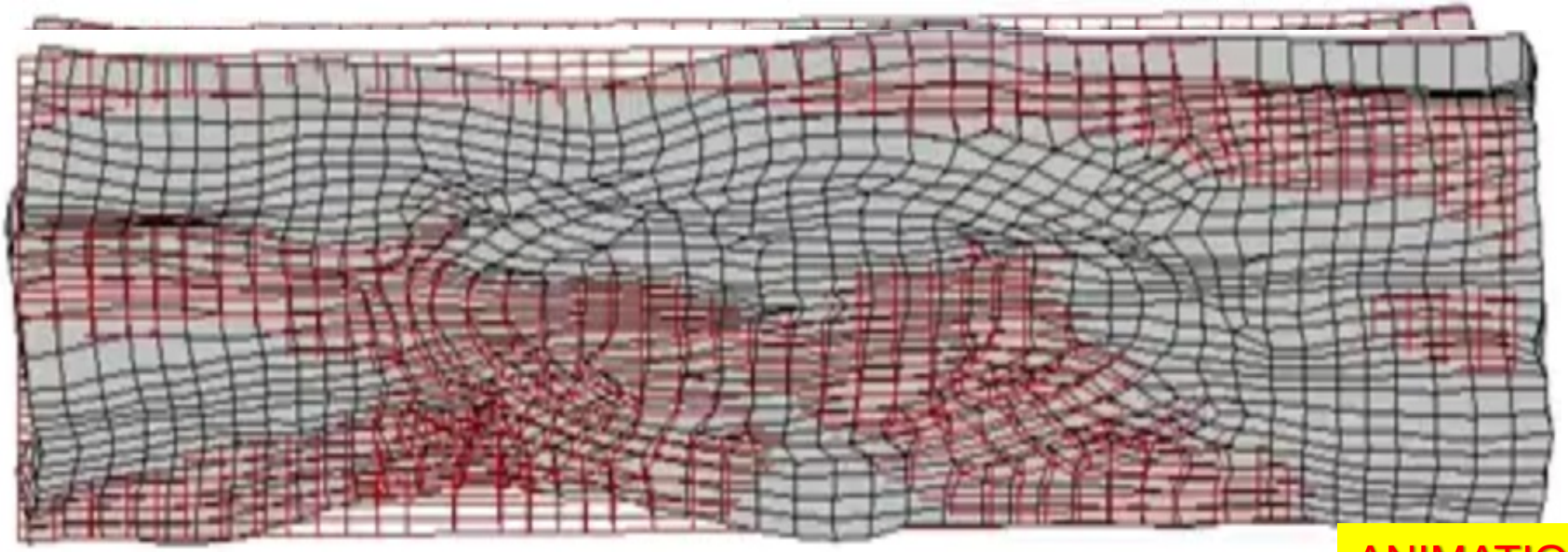
Incoherency Simulation With ATF Zero-Phasing (Loss of Physics)



Incoherency Simulation With Random Phasing (No Loss of Physics)



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ANIMATIONS

Effects of Foundation Size on SSI Responses

ASCE 04-1998 and SRP 3.7.2 Requirements

TABLE 3.3-2. Reductions to Ground Response Spectra

Frequency (Hz)	Reduction Factor for Plan Dimension of 150 ft	Reduction Factor for Plan Dimension of 300 ft
5	1.0	1.0
10	0.9	0.8
≥25	0.8	0.6

SRP 3.7.2
0.0/10Hz
0.7/30Hz
for all foundation sizes

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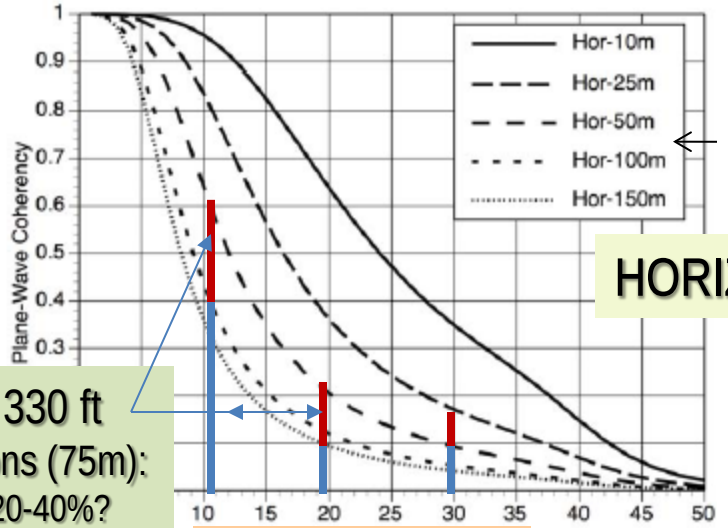
2007 Abrahamson Coherence for Hard-Rock and Soil Sites



420 ft x 330 ft
Reductions (75m):
At 10 Hz 20-40%?
At 20 Hz 50-70%?

Figure 6-1
Plane-Wave Coherency for the Horizontal Component

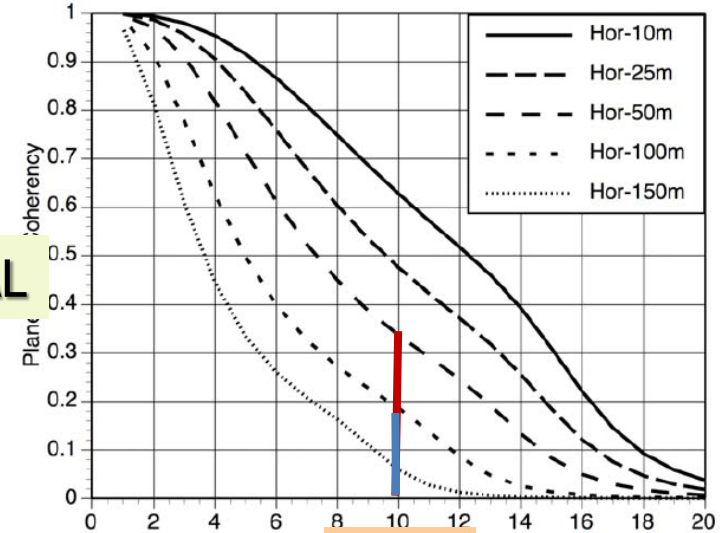


Figure 7-1
Plane-Wave Coherency for the Horizontal C

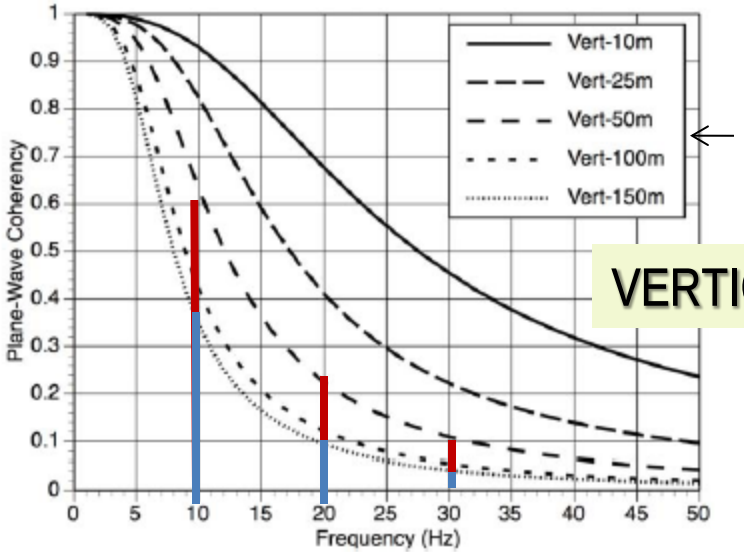


Figure 6-2
Plane-Wave Coherency for the Vertical Component

Larger differential displacements for soils. May affect SSSI..?

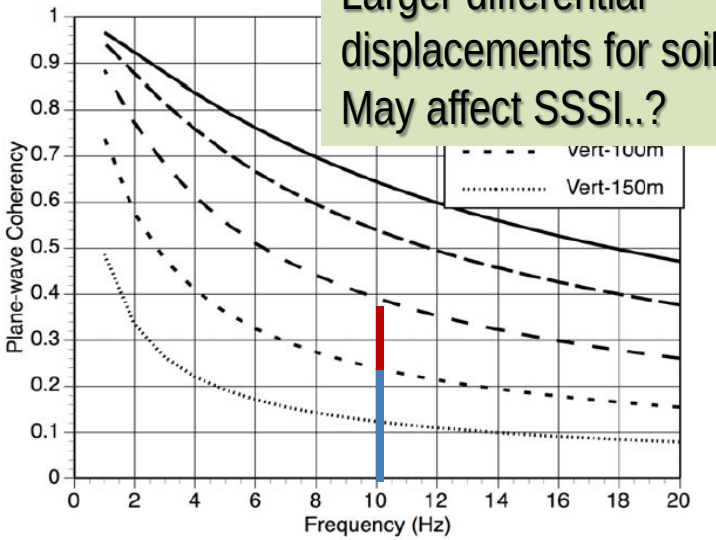


Figure 7-2
Plane-Wave Coherency for the Vertical Component for Soil Sites

(EPRI TR # 1015110, December 2007)

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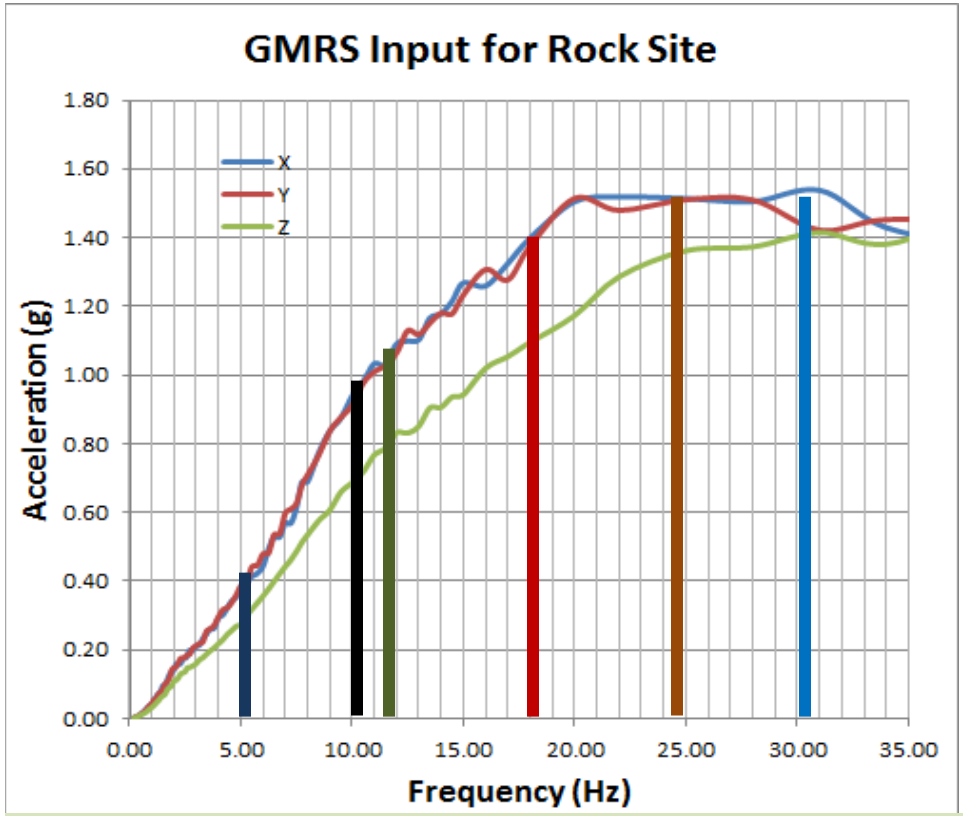
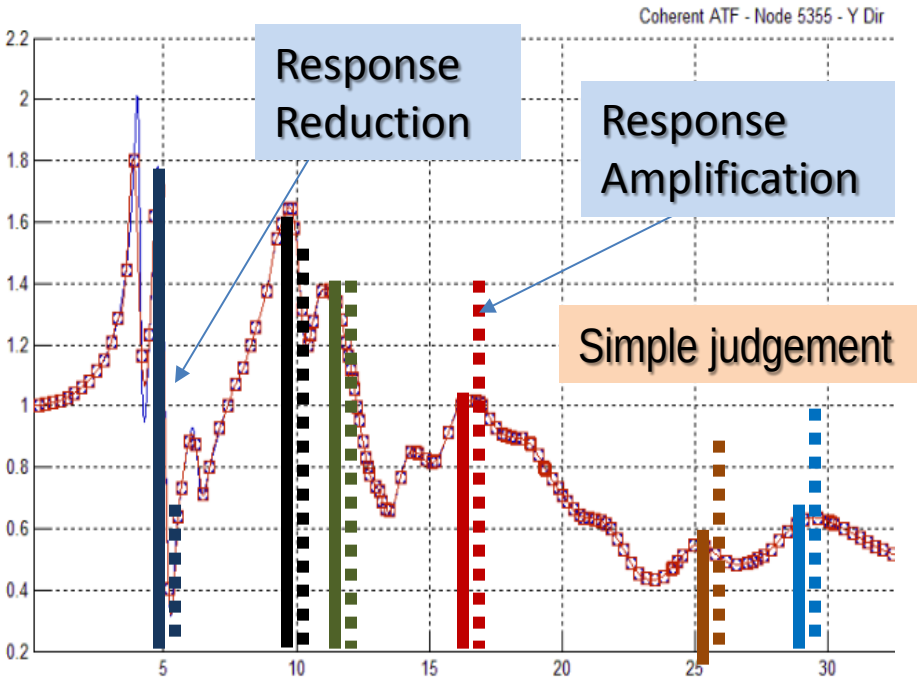
Incoherency Effects on Multiple Mode SSI Responses

Seismic Motion Amplitude Shape

ATF Amplitude Spectral Shape





Remark: For the above GMRS, the ISRS at the selected location will be dominated by the 10 Hz, 12 Hz and 17 Hz mode responses. The 5 Hz component will be much lower. Incoherent ISRS reductions will correspond to a mix of components in the 10-17 Hz range.

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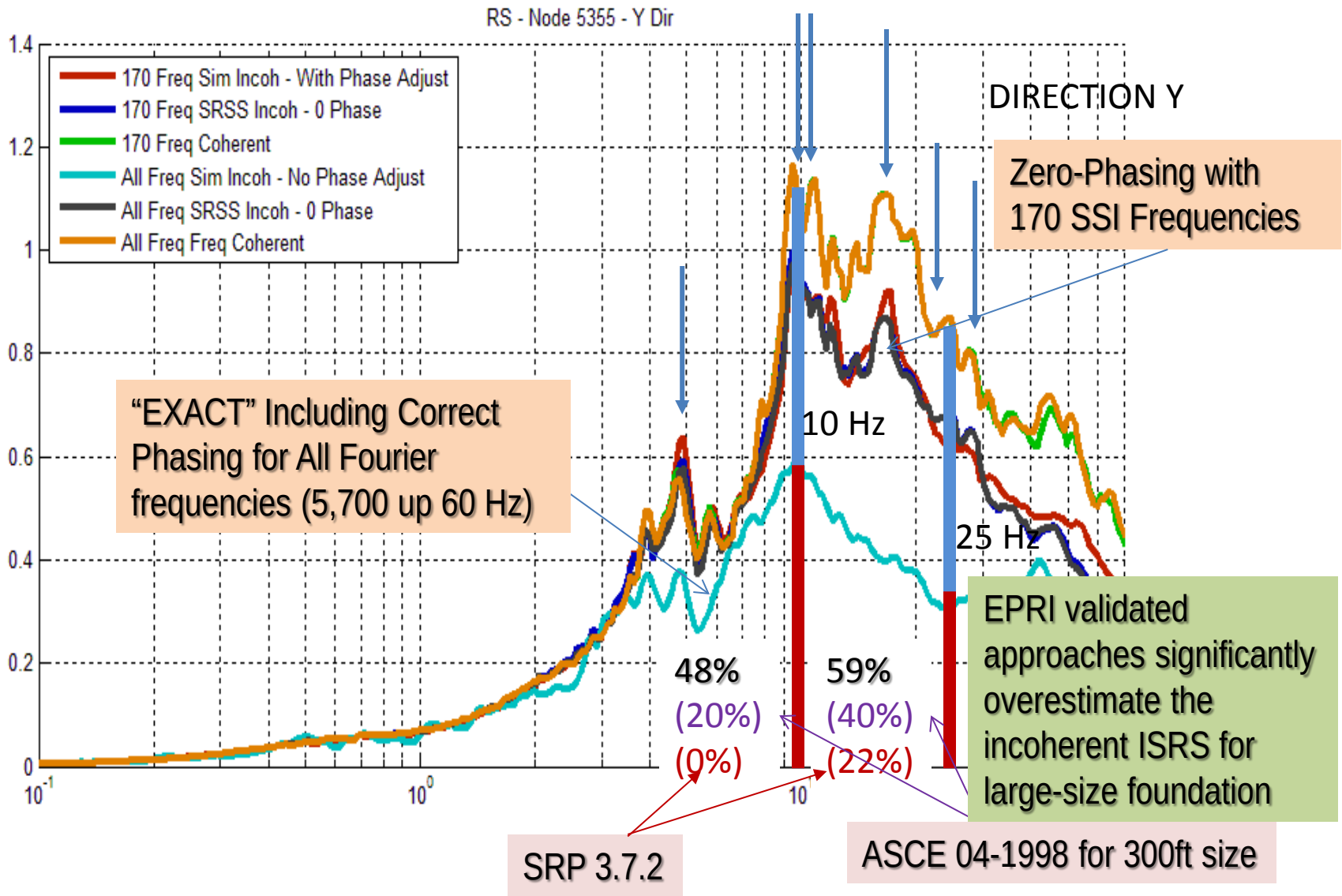
Effects of Random Phasing on ISRS for Large-Size 420 ft x 330 ft RB Complex



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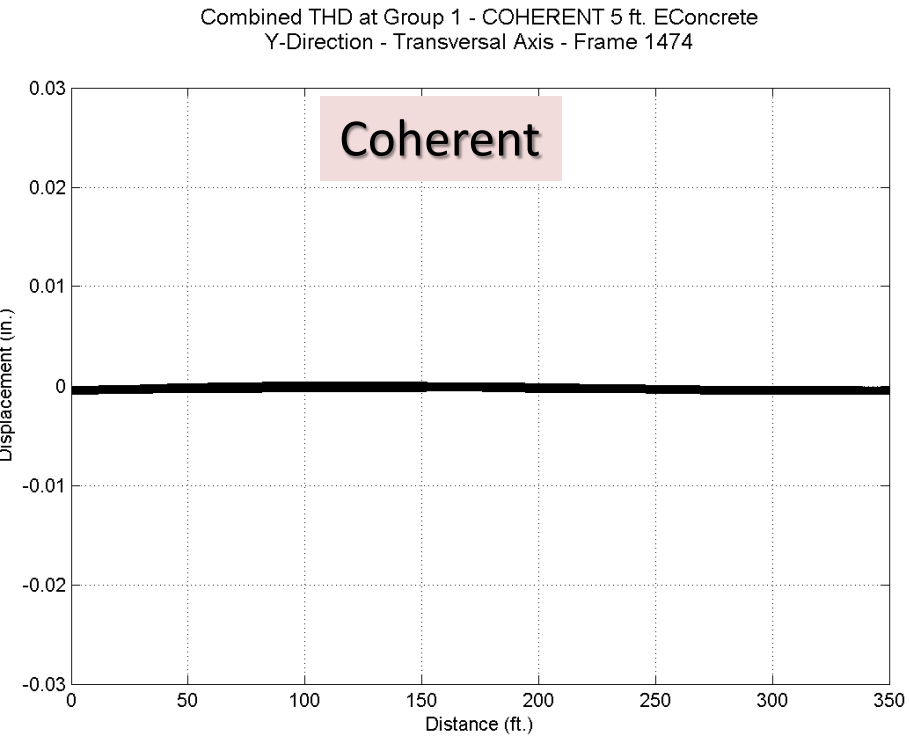
Effects of Incoherency on Basemat Bending



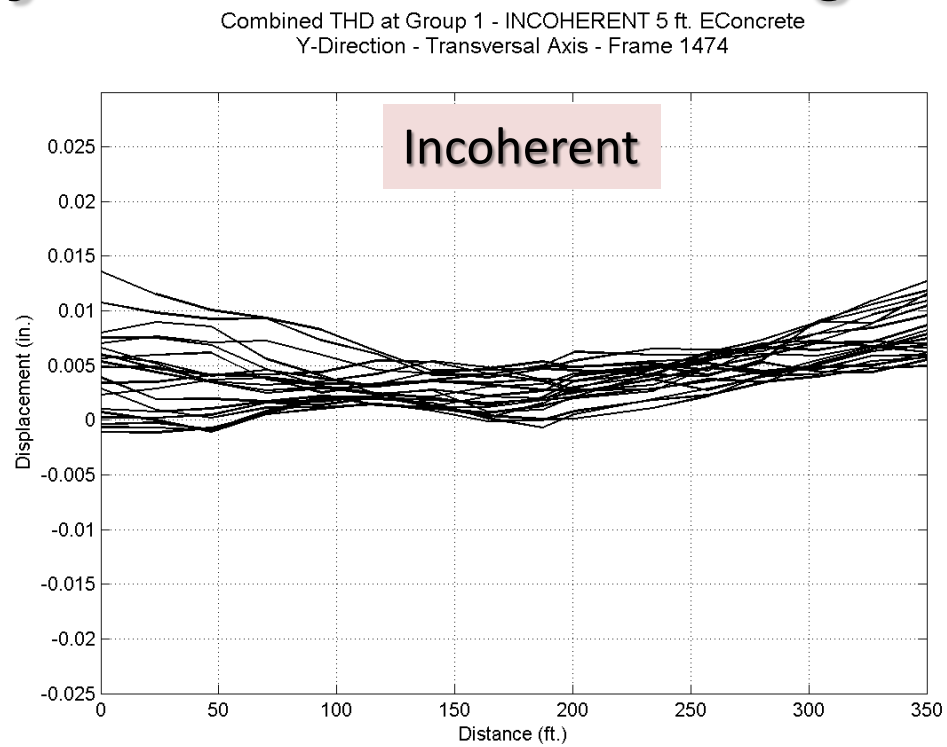


Table 1: Baseslab Bending Moments for A Soil Deposit with Vs = 3,300 ft/s

Zone #	Coherent MXX	Incoherent MXX	Ratio Inc./Coh MXX	Coherent MYY	Incoherent MYY	Ratio Inc./Coh MYY
1	10.293	15.196	1.476	9.567	14.812	1.548
2	8.345	19.986	2.395	7.197	14.901	2.070
3	10.291	13.499	1.312	9.695	15.475	1.596
[illegible]	[illegible]	.859	2.007	8.386	17.199	2.051
[illegible]	[illegible]	.618	1.986	7.124	14.879	2.089
[illegible]	[illegible]	.503	2.375	8.354	14.293	1.711

Remark: Incoherent bending moments are 130%-240% of coherent bending moments.

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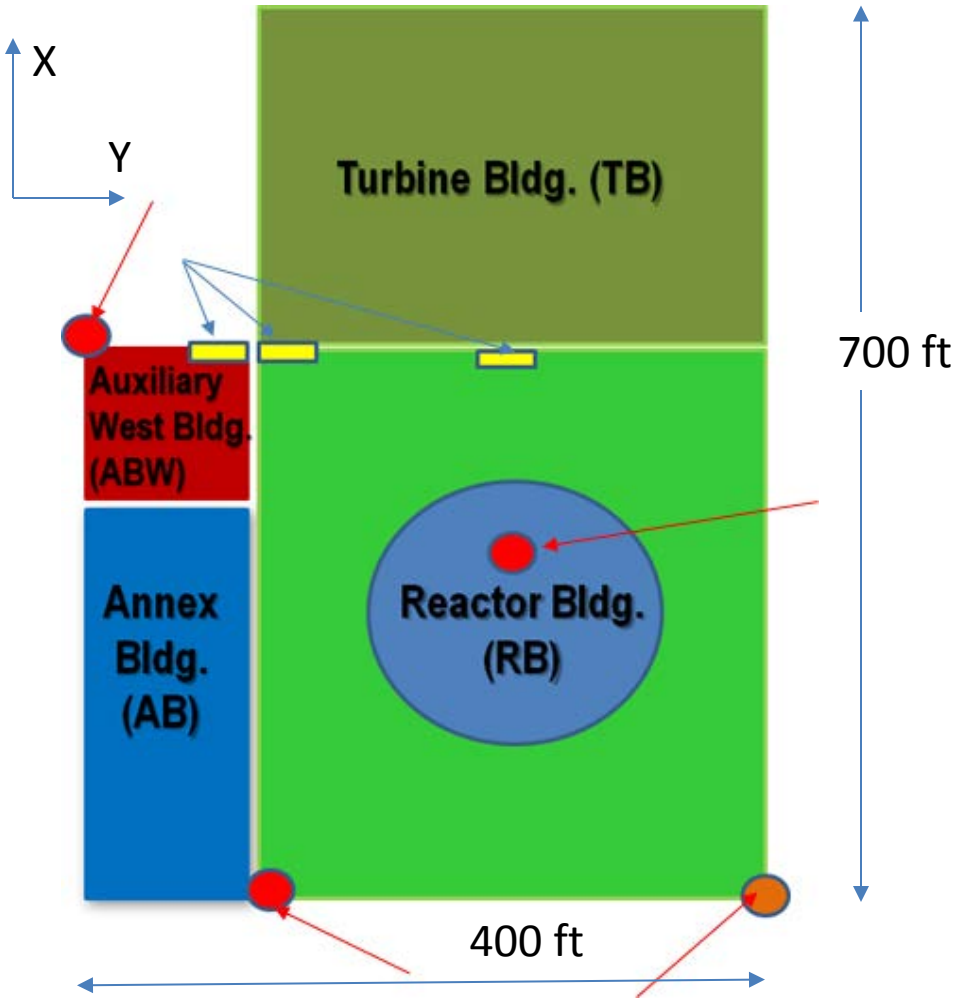
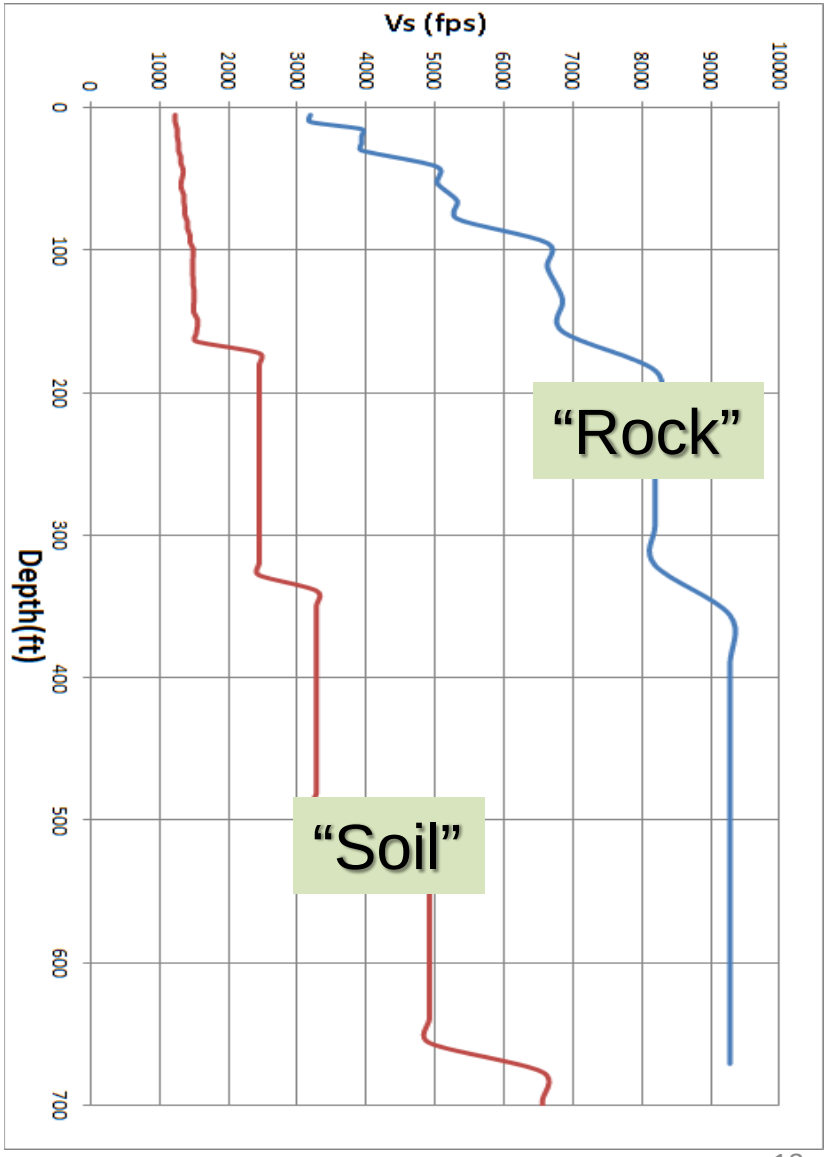
Incoherent vs. Coherent Seismic SSSI Effects

Generic NPP SSSI Model 1

(55,000 nodes with 5,000 int. nodes, 27,000 shells, 13000 solids, 11000 beams)



Soil Profiles



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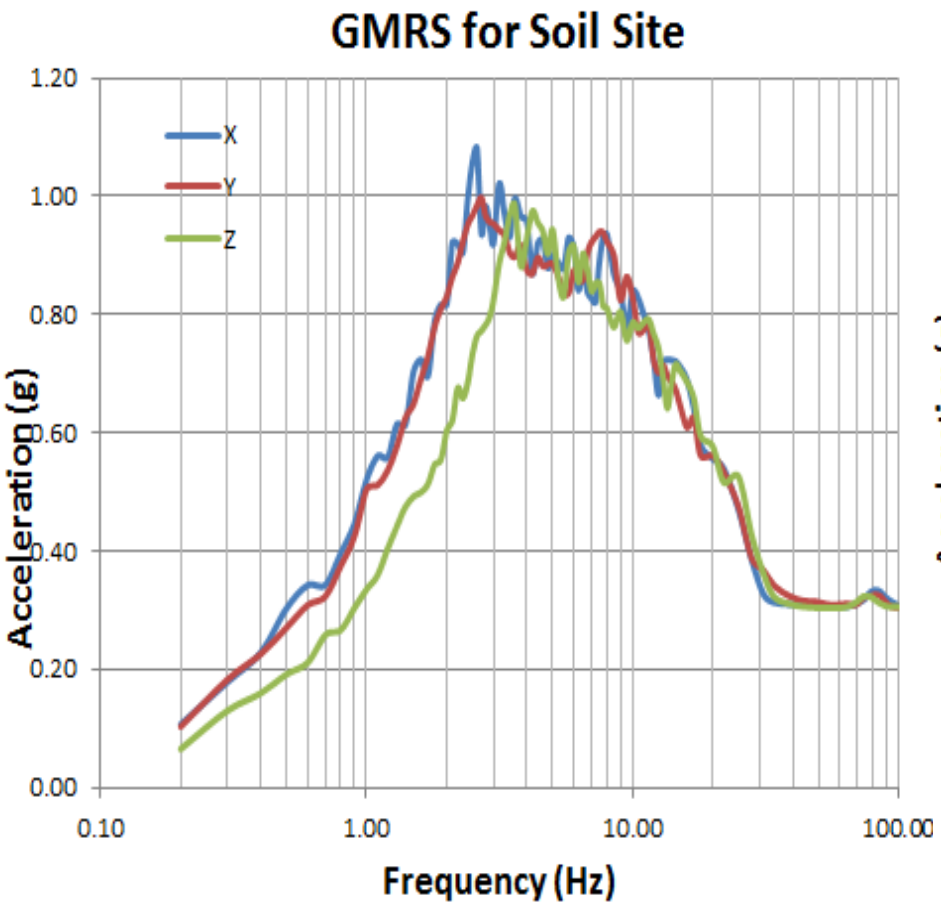
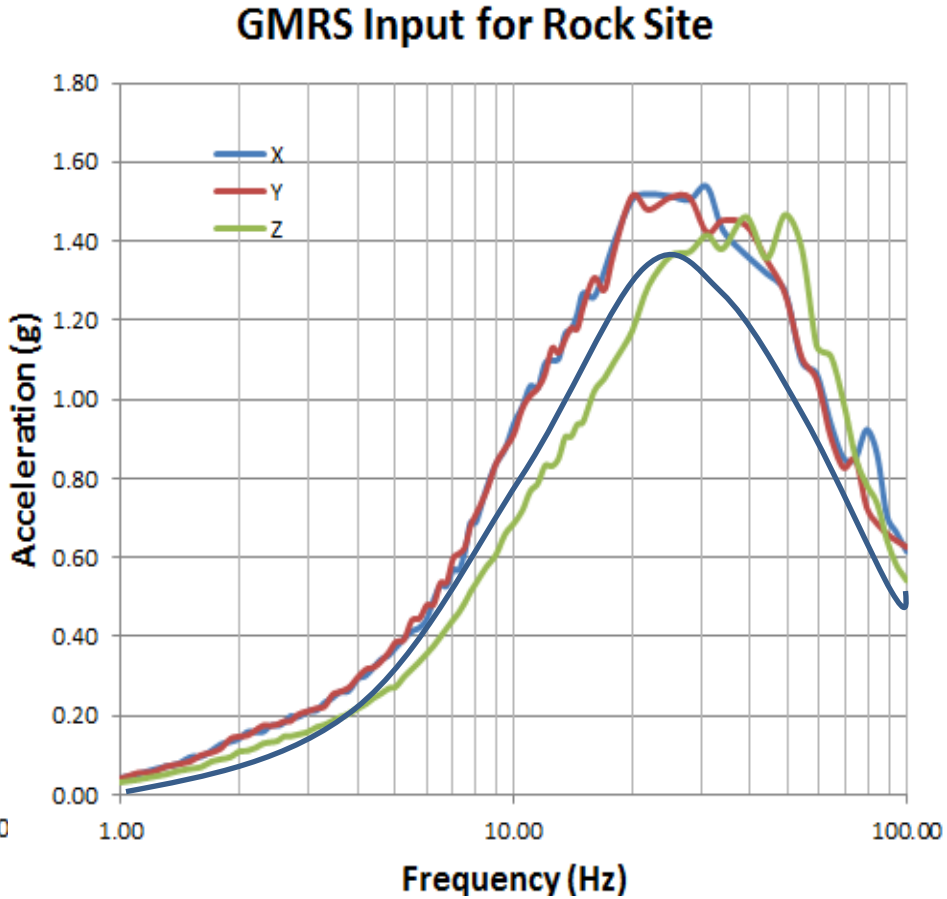
Seismic GMRS Surface Input for Soil and Rock Sites





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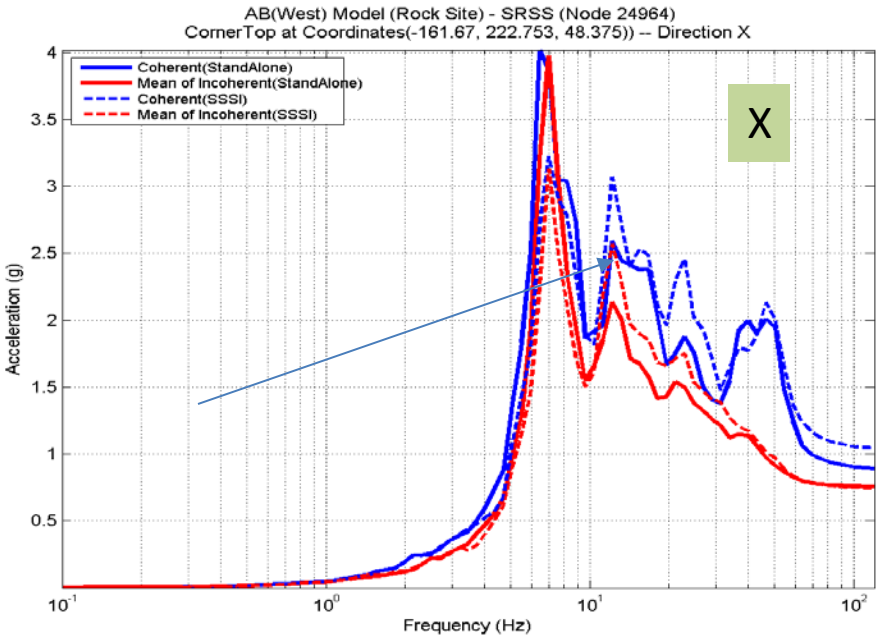
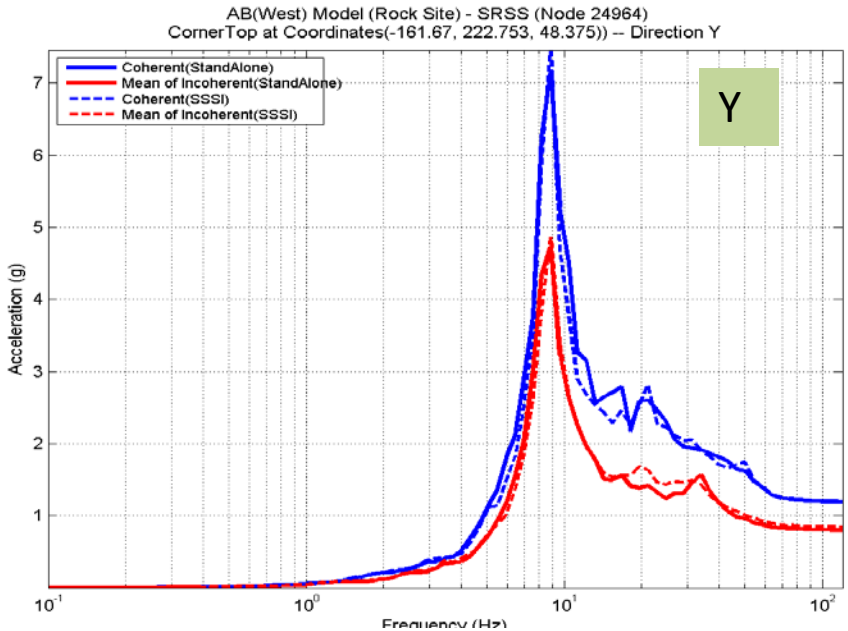
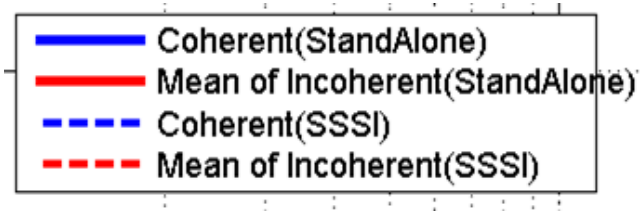
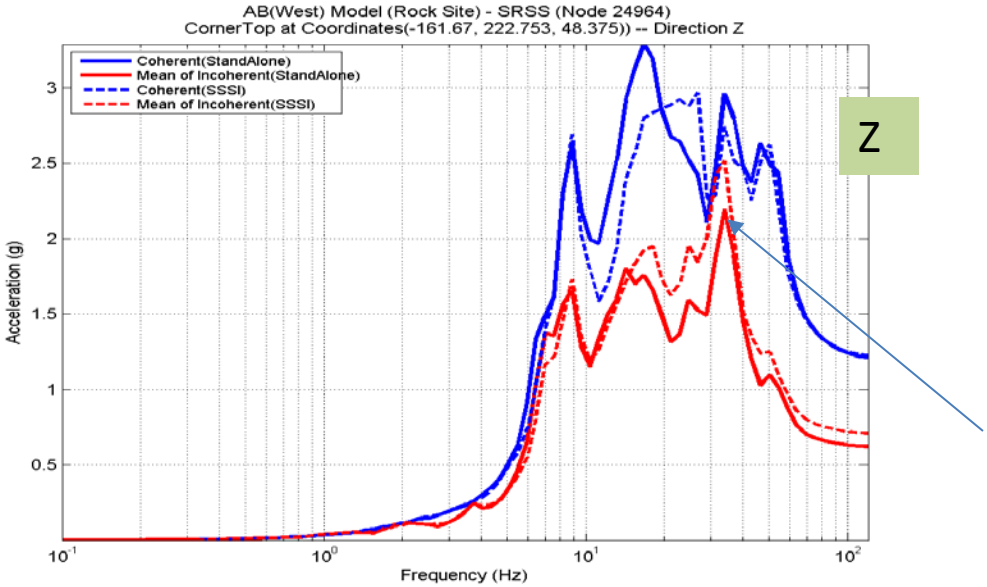
ABW Bldg. Coherent vs. Incoherent SSI and SSSI Effects for Roof Corner ISRS for Rock Site









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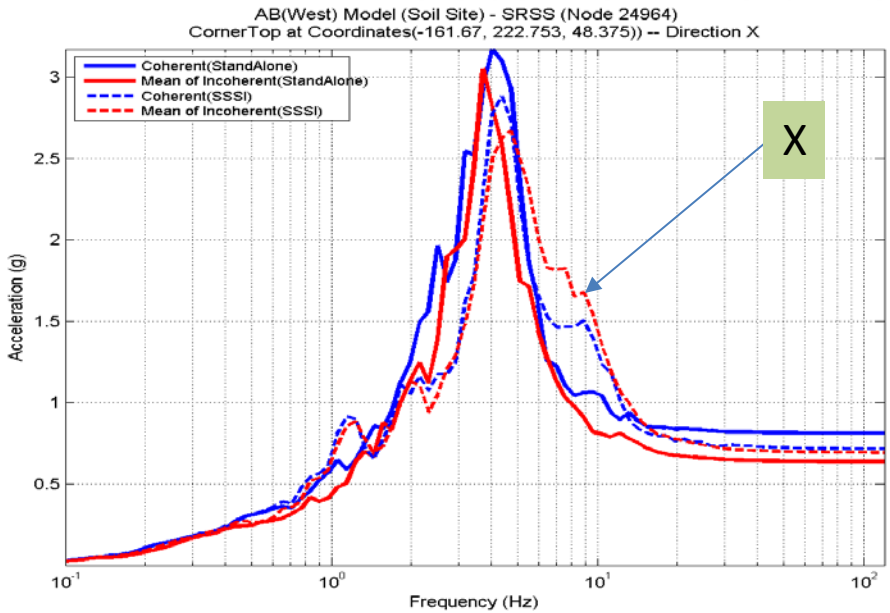
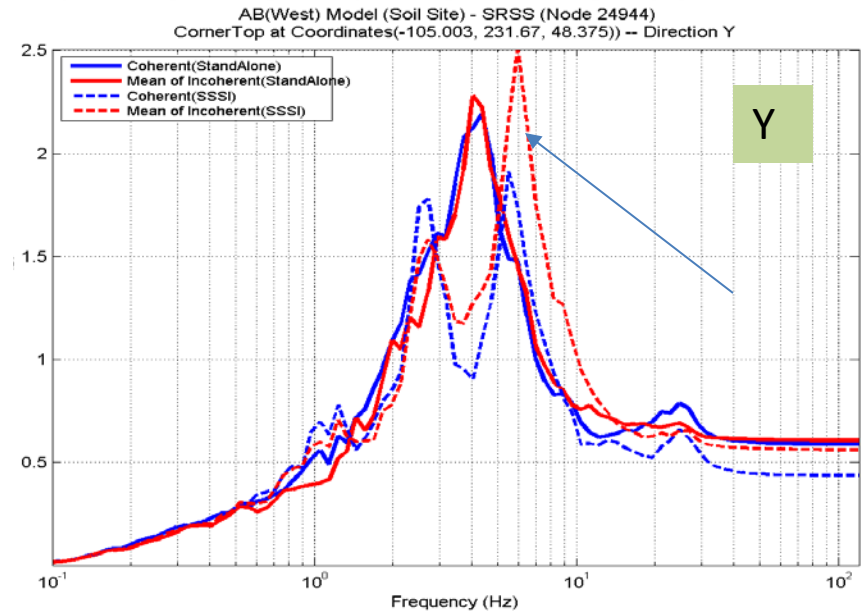
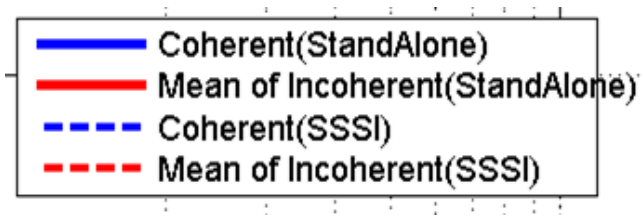
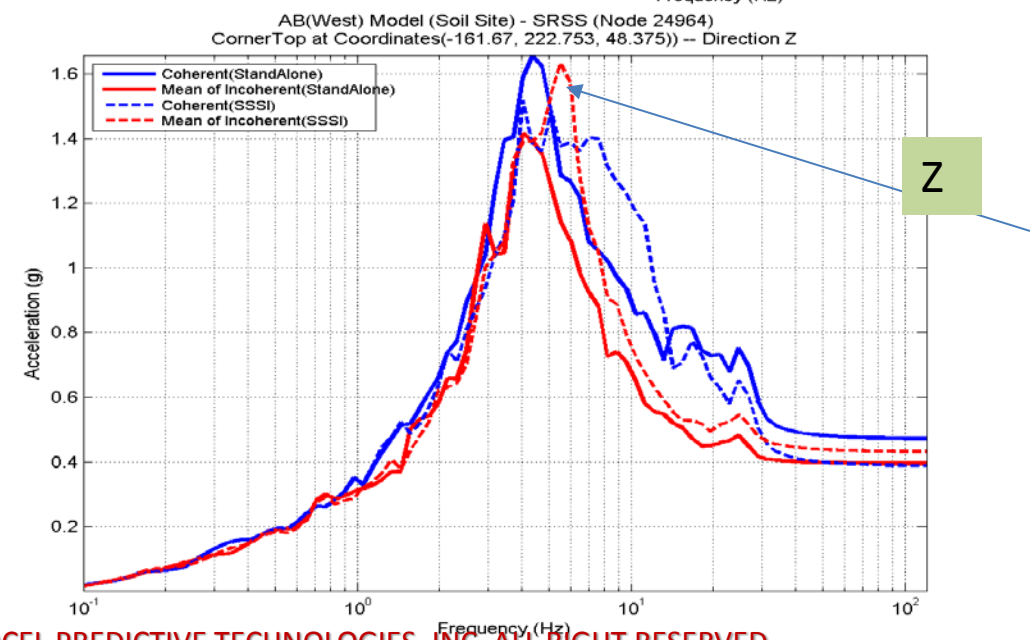
ABW Bldg. Coherent vs. Incoherent SSI and SSSI Effects for Roof Corner ISRS for Soil Site









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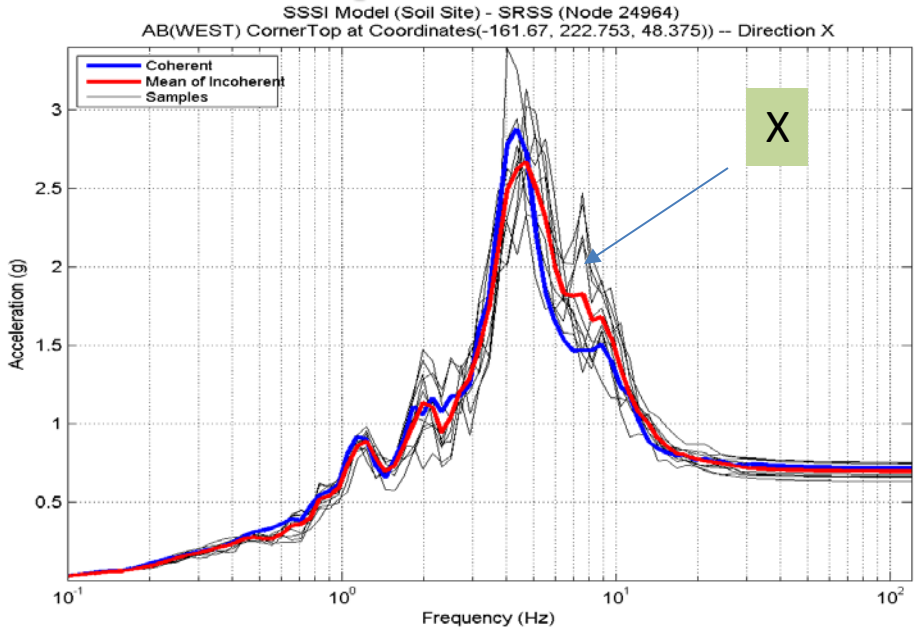
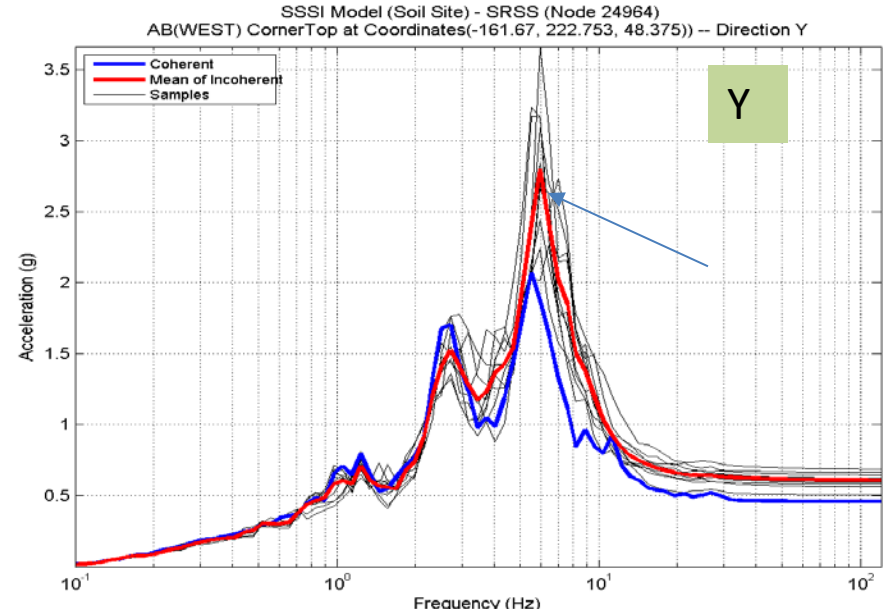
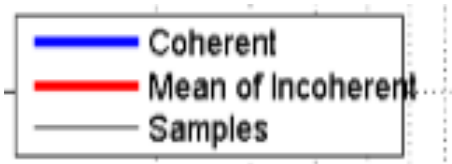
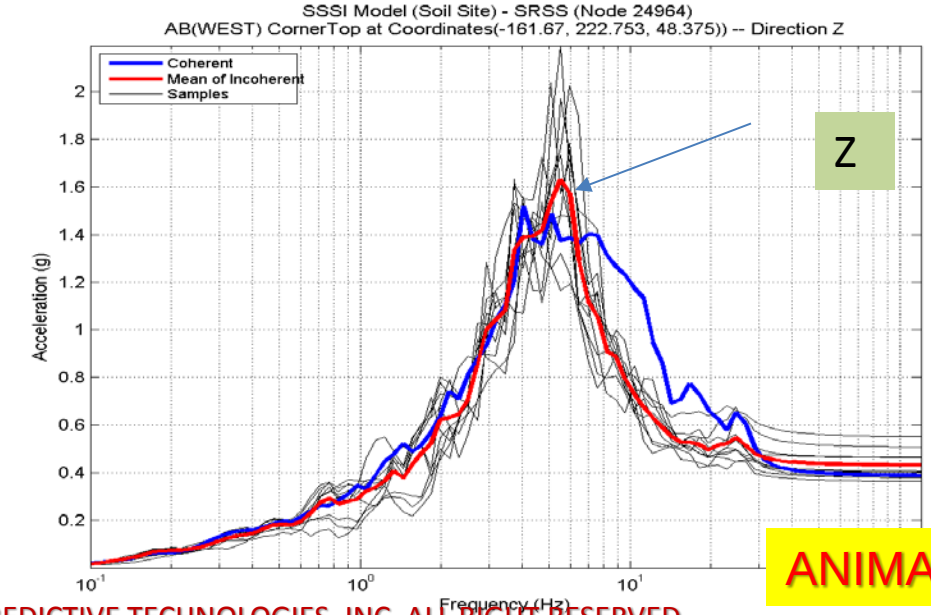
ABW Bldg. Coherent vs. Incoherent SSSI Simulated Responses for Roof Corner ISRS for Soil Site









ANIMATIONS

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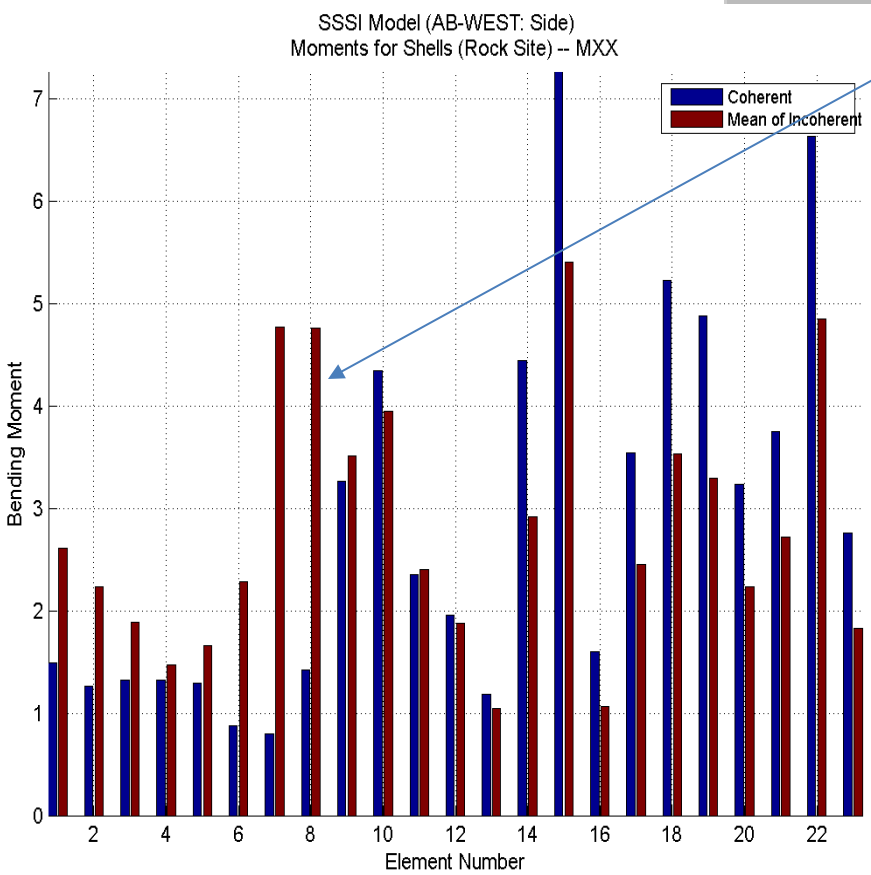
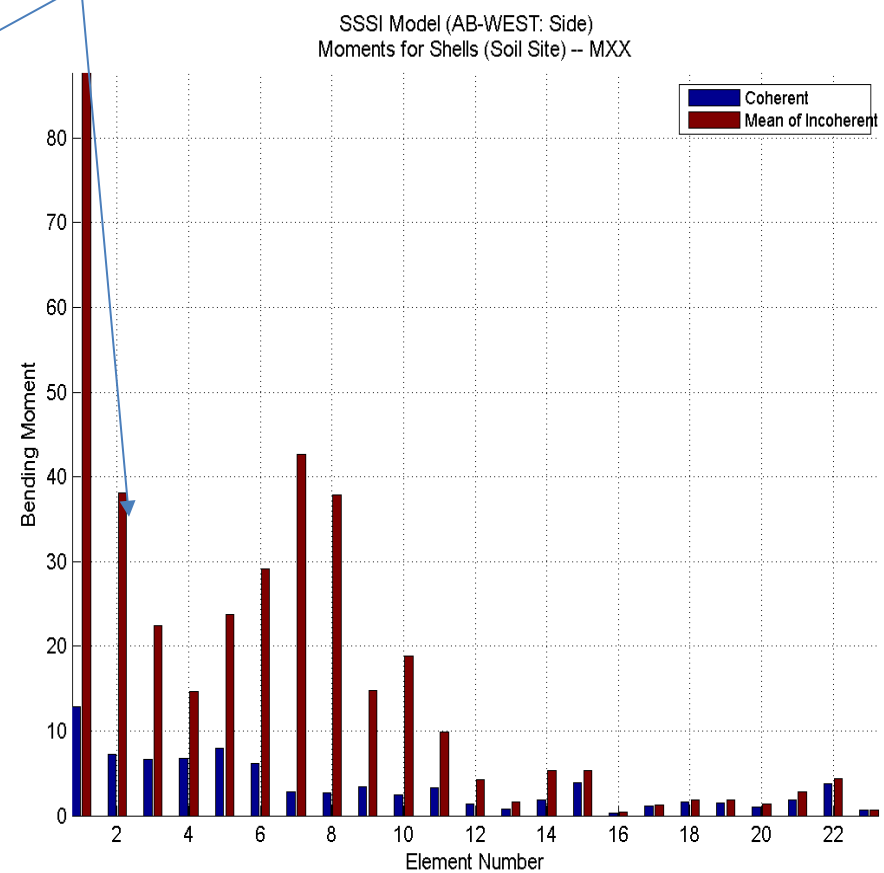
ABW Bldg. Coherent vs. Incoherent SSSI Effects on Bending Moments in Corner Walls Near RB Complex

Rock Site

Soil Site





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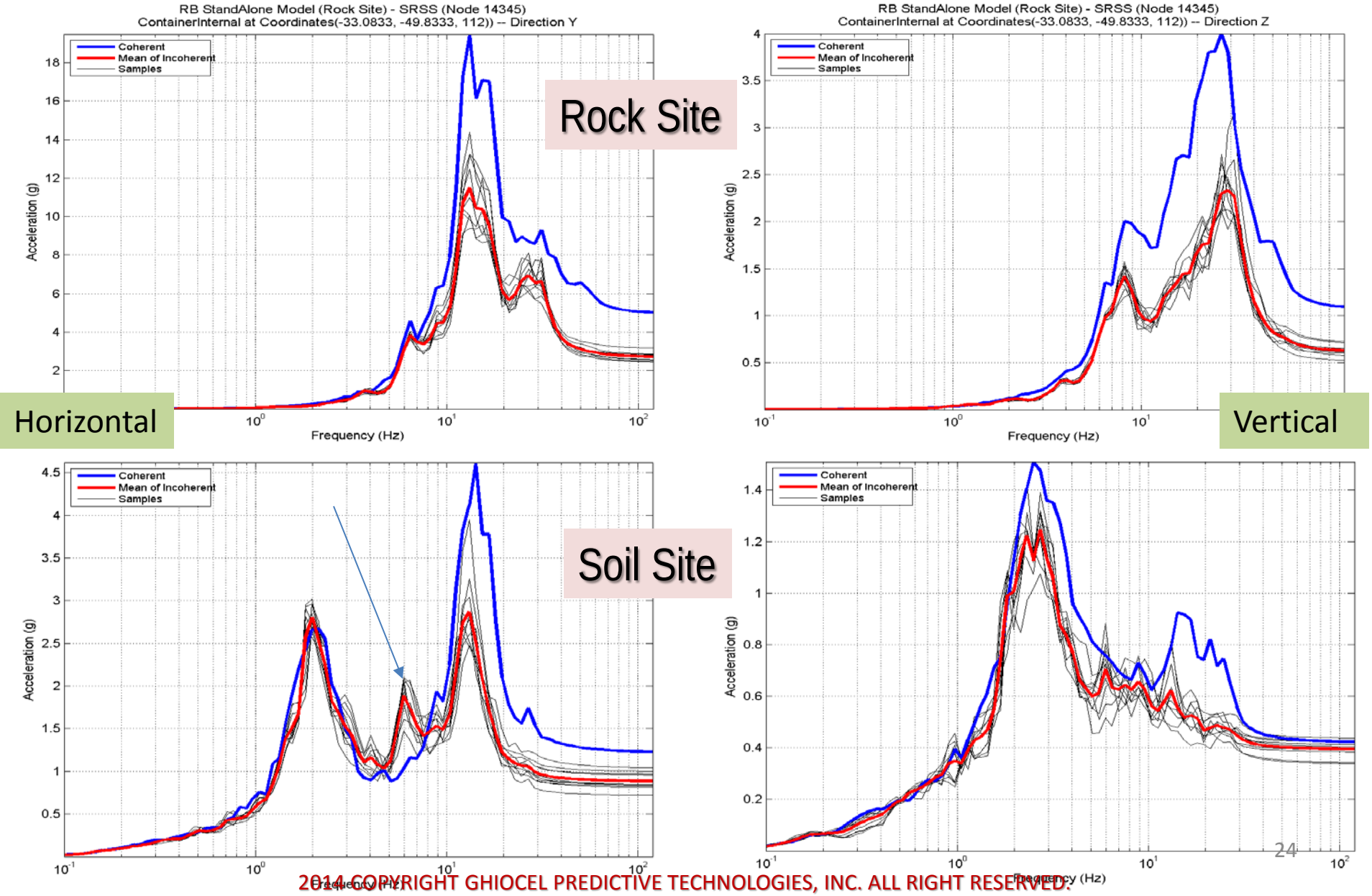
RB Complex Coherent vs. Incoherent SSSI Effects on ISRS on Top of Internal Structure – Y and Z Directions



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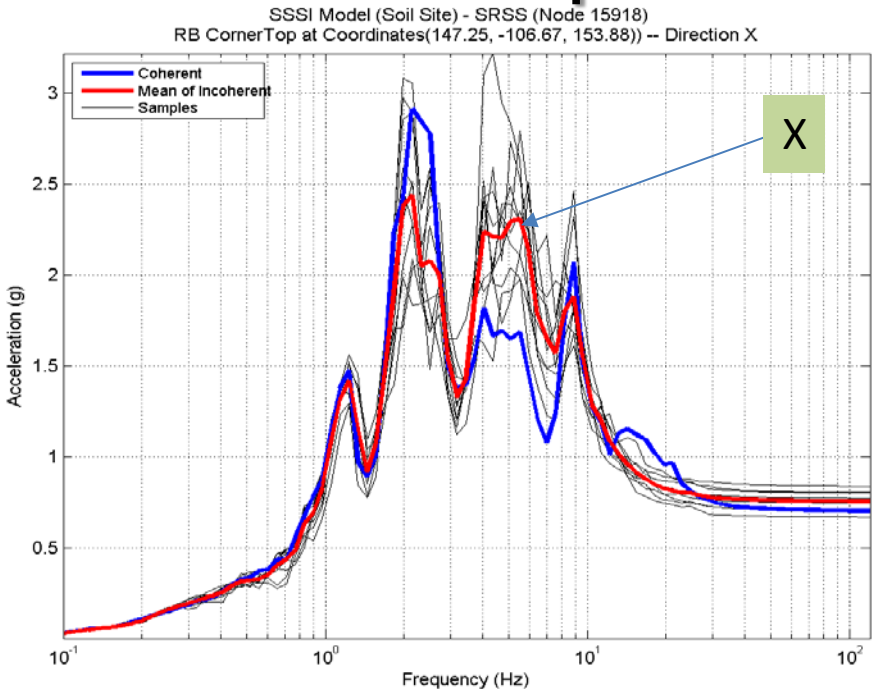
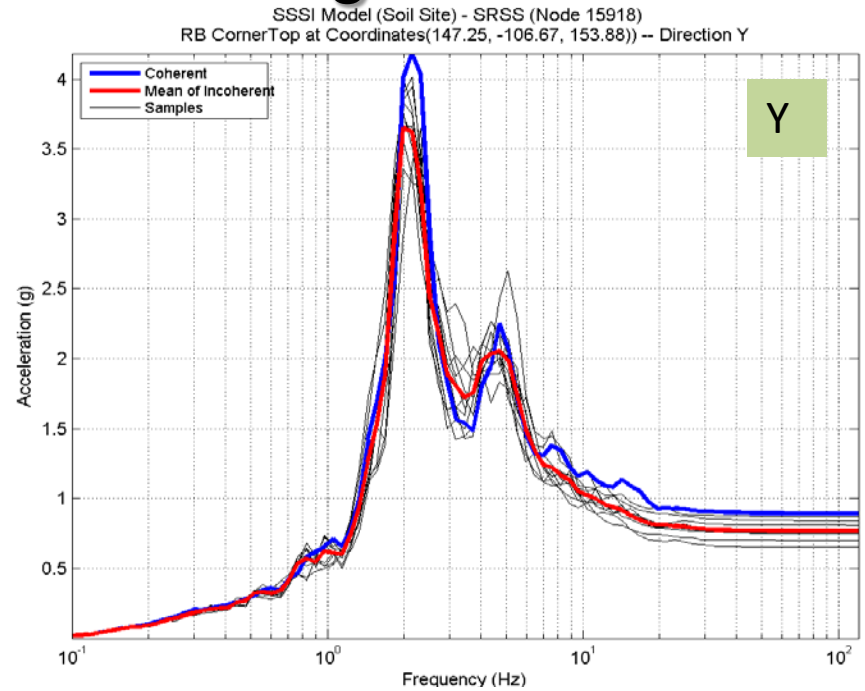
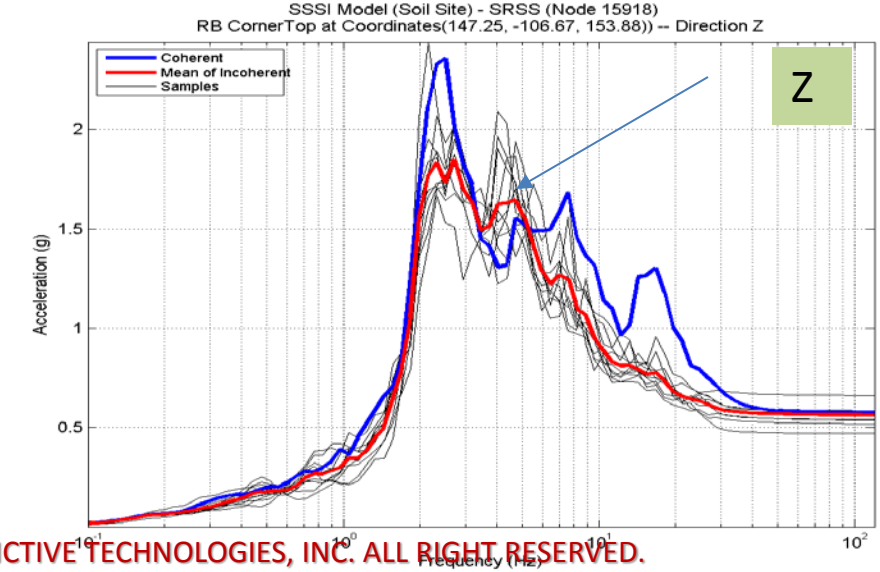
RB Complex Coherent vs. Incoherent SSSI Effects on ISRS at Top Corner Near AB Bldg. for Soil Site







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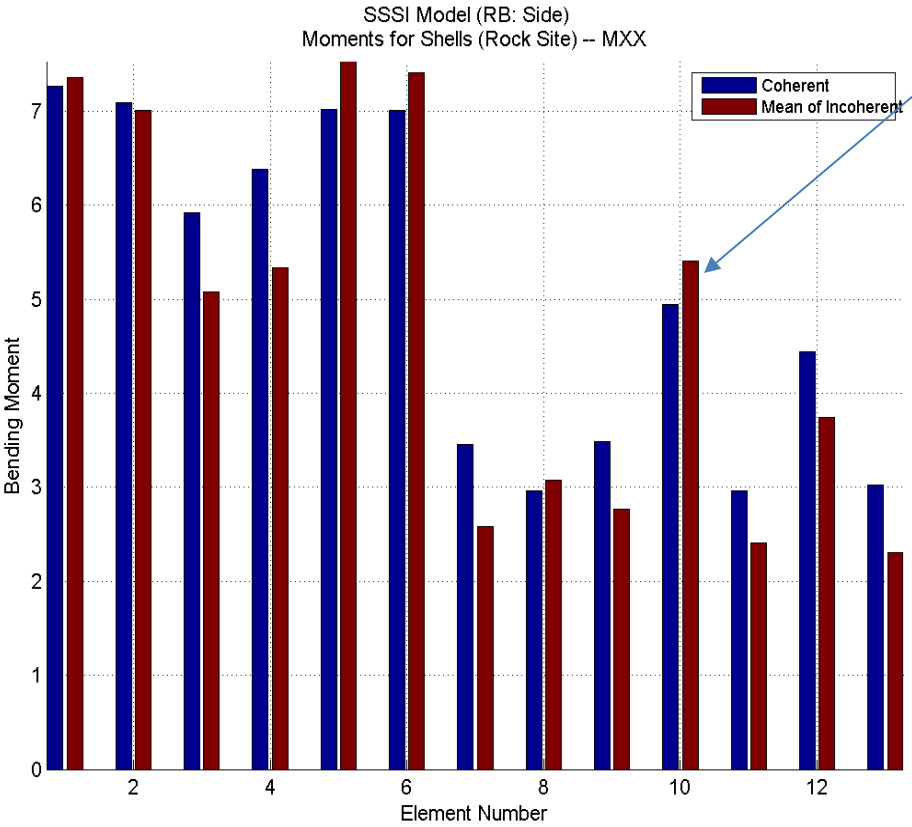
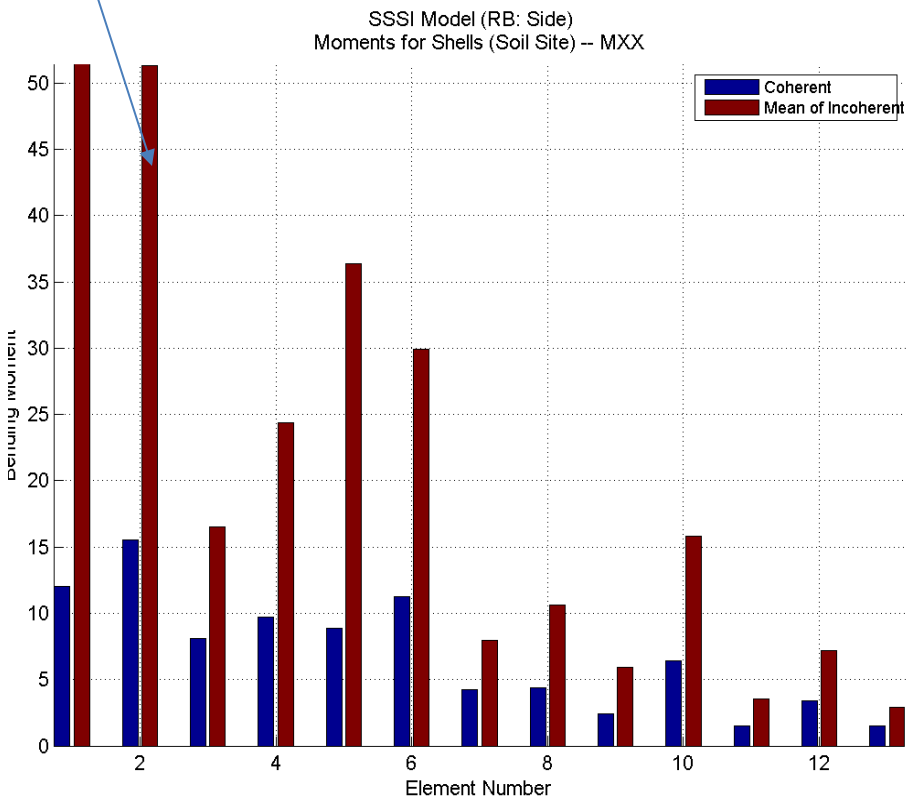
RB Complex Coherent vs. Incoherent SSSI Effects on Bending Moments in Corner Wall Near ABW Bldg.

Rock Site

Soil Site





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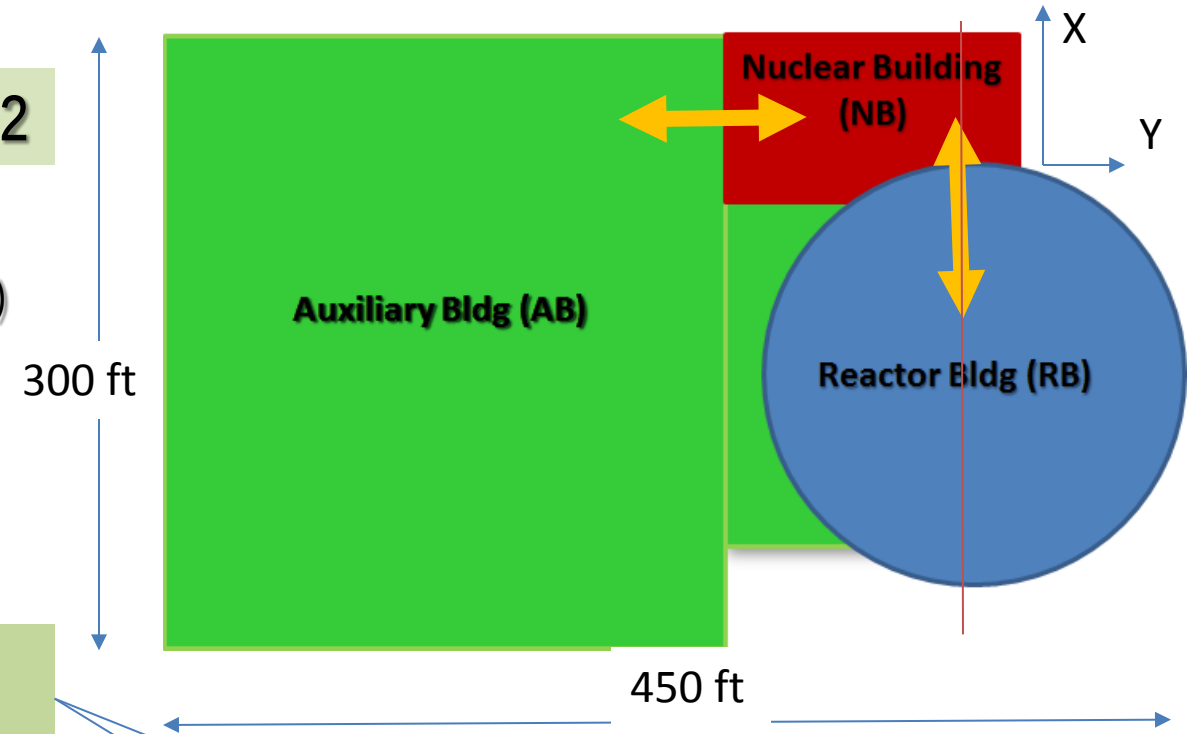
Incoherent vs. Coherent Seismic SSSI Effects

Generic NPP SSSI Model 2

(75,000 nodes with 11,000 int. nodes,45,000 solids, 11,000 shells)

Compute relative displacements between NB and RB buildings:

Differential motion amplitude is twice larger for incoherent input



		Displacements									Absolute Difference of Displacements					
		Coordinates			Coherent			Incoherent-Sample1			Coherent			Incoherent-Sample1		
Node Number	Model	X	Y	Z	X	Y	Z	X	Y	Z	X	Y	Z	X	Y	Z
49678	RB	-75.615	27	30	0.012455	0.012657	0.004428	2.22419	2.62085	1.51993						
49760	FHB	-76.67	26.922	30	0.073812	0.033553	0.152572	2.35583	2.65359	1.65843	0.061357	0.020896	0.148144	0.13164	0.03274	0.1385
14966	RB	-72.881	33.75	30	0.012455	0.012657	0.004428	2.22419	2.62085	1.51993						
32740	FHB	-73.941	33.611	30	0.072556	0.035056	0.133265	2.35014	2.65559	1.62887	0.060101	0.022399	0.128837	0.12595	0.03474	0.10894

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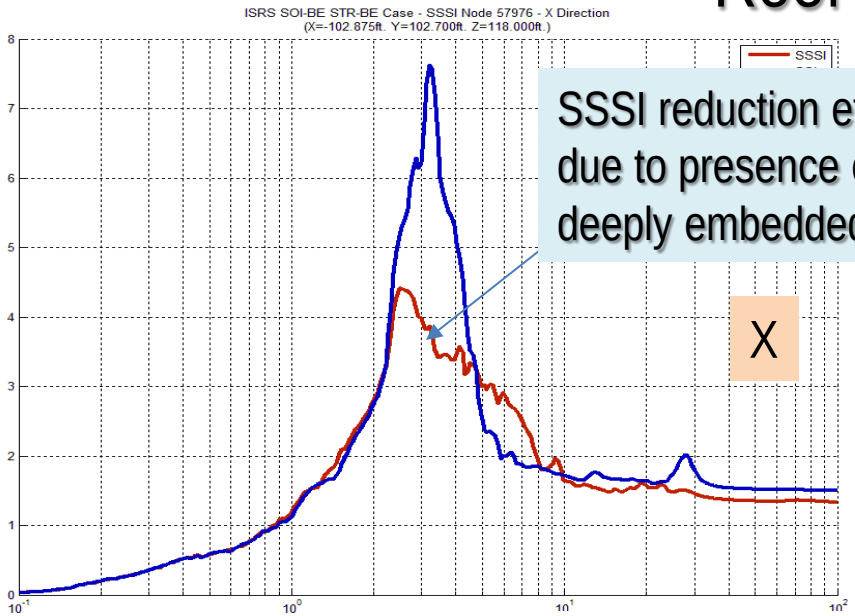
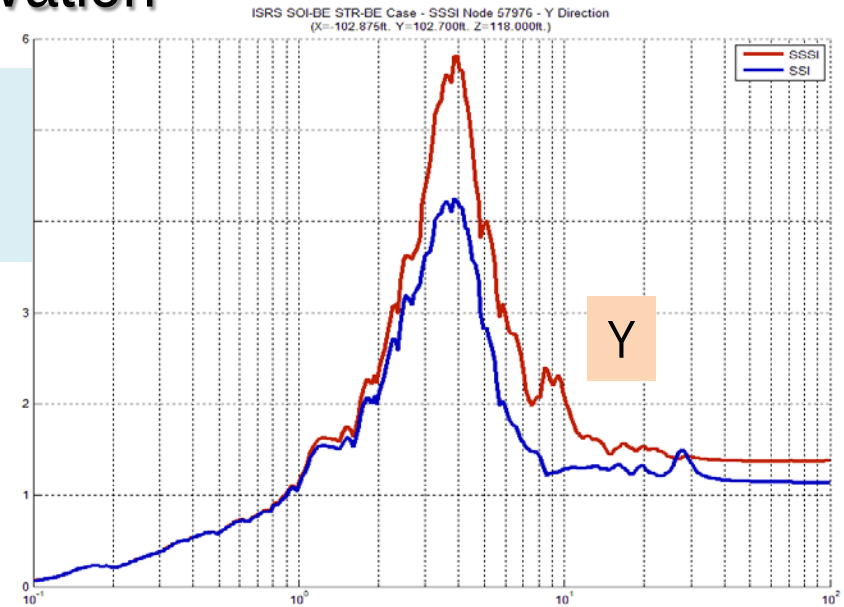
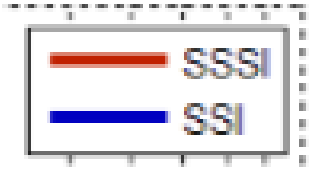
Seismic SSSI Effects on the Adjacent NB ISRS

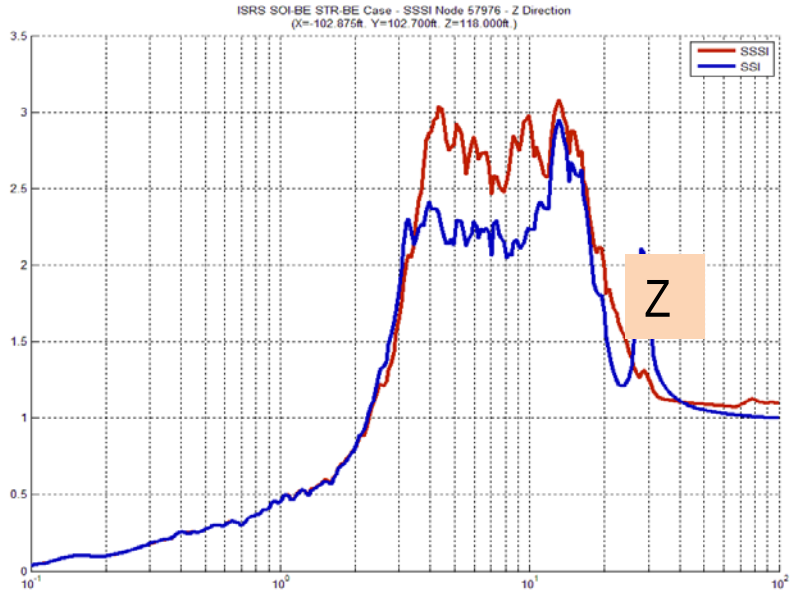
Roof Elevation



SSSI reduction effects due to presence of deeply embedded RB







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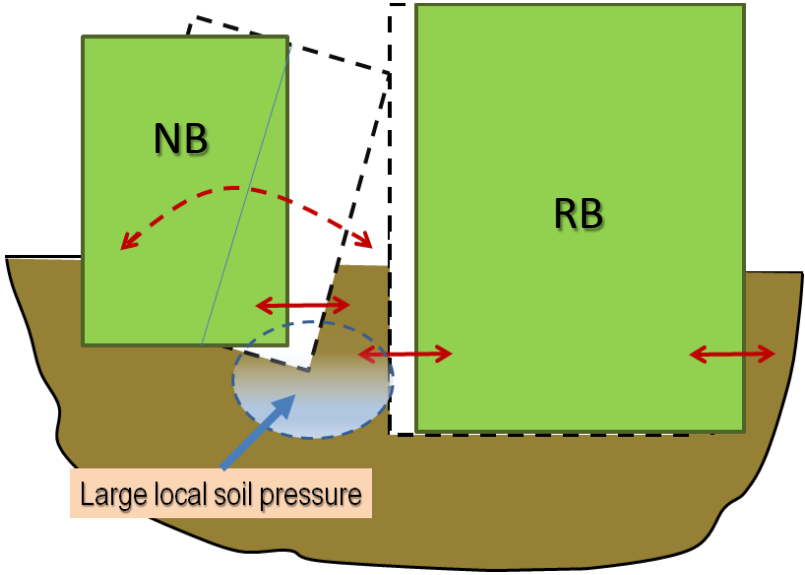
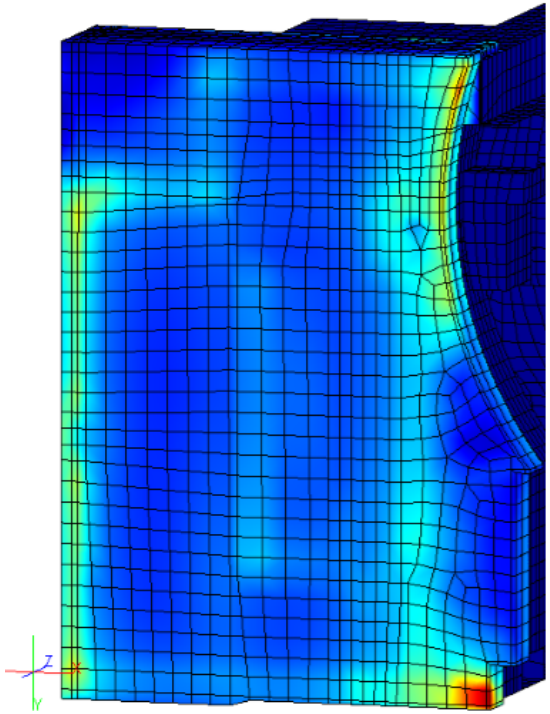
Seismic SSSI Effects on NB Basemat Pressures

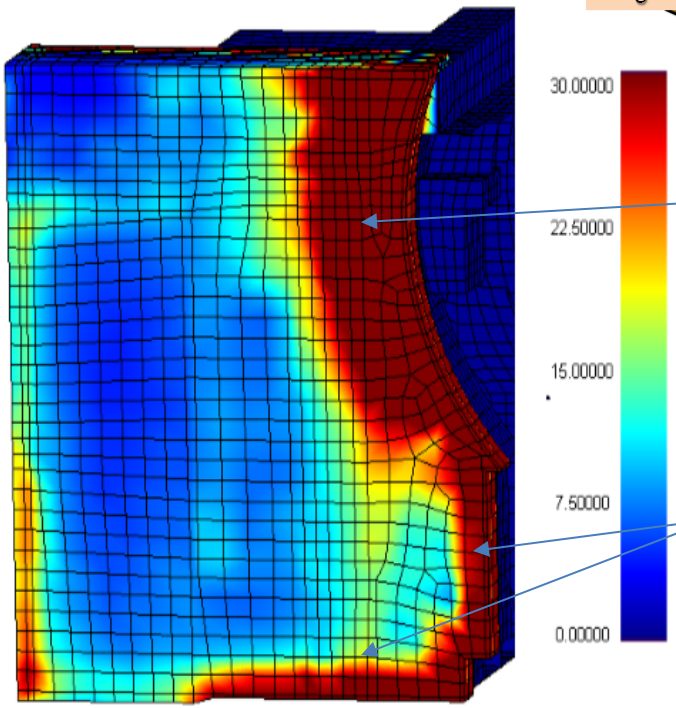
SSSI Effects may increase severely seismic pressures on foundation walls and basemat. Suggestion to include local nonlinear soil behavior (only 2-3 iterations)



SSI Pressures

SSSI Pressures





SSSI Effects due to RB

SSSI Effects due to AB

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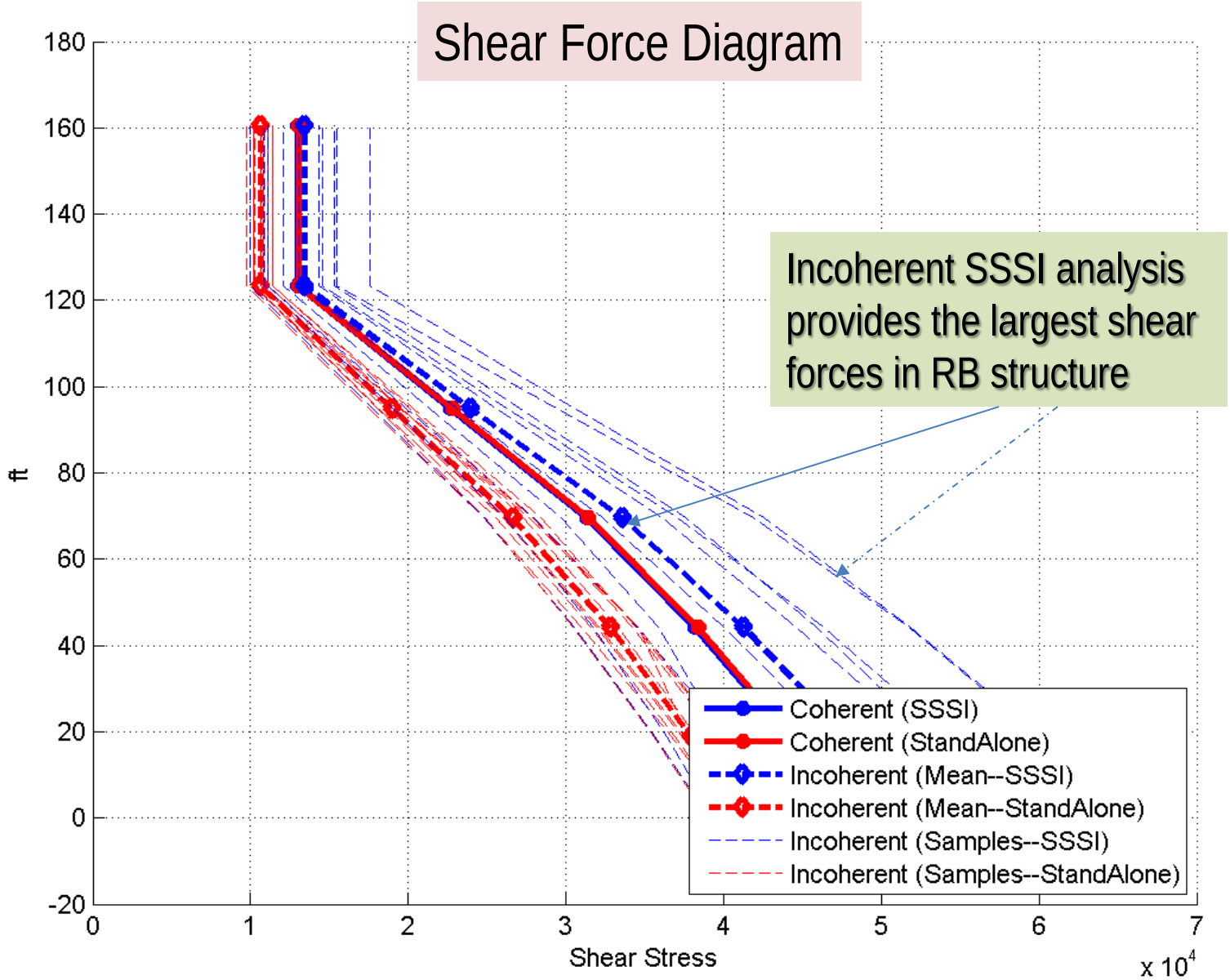
Seismic SSSI Effects on Shear Forces in RB



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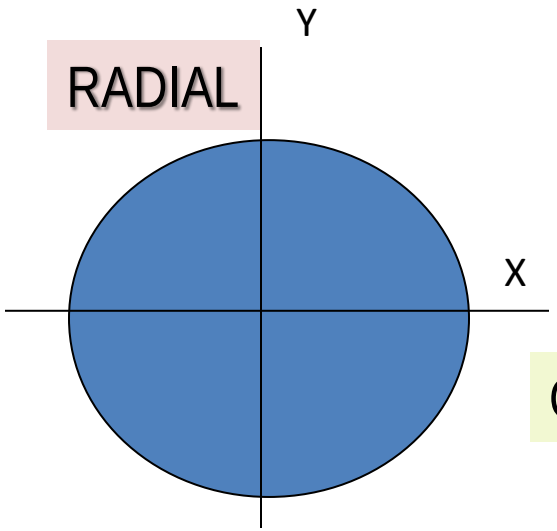
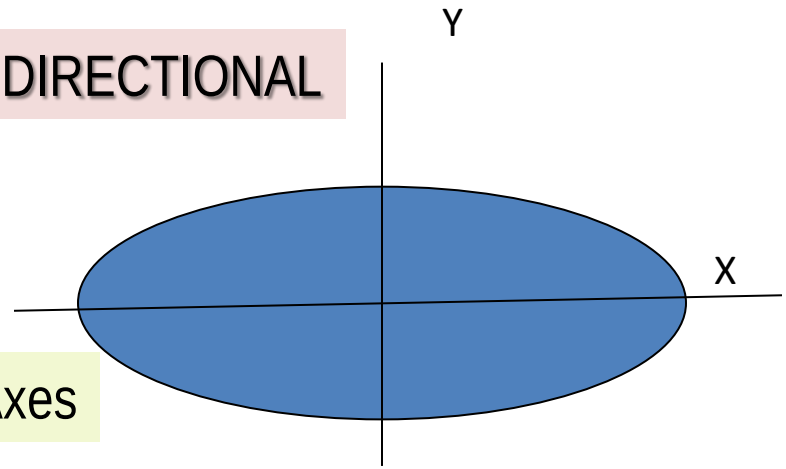
Abrahamson Radial vs. Directional Coherency Models

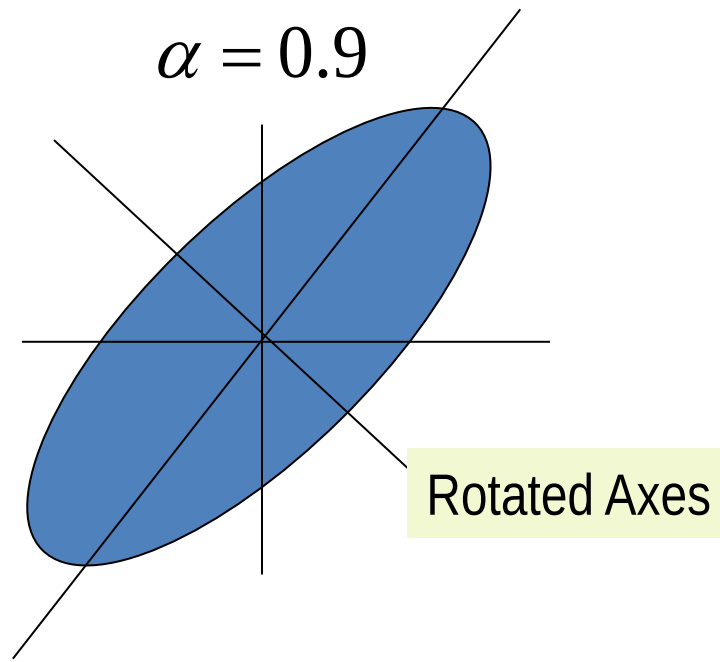


$\alpha = 0.5$



Global Axes

$\alpha = 0.9$



Incoherency distance is

$$D_{\vee}\zeta = \zeta[(1-\alpha)D_{x\vee}\zeta + \alpha D_{\lambda\vee}\zeta]$$

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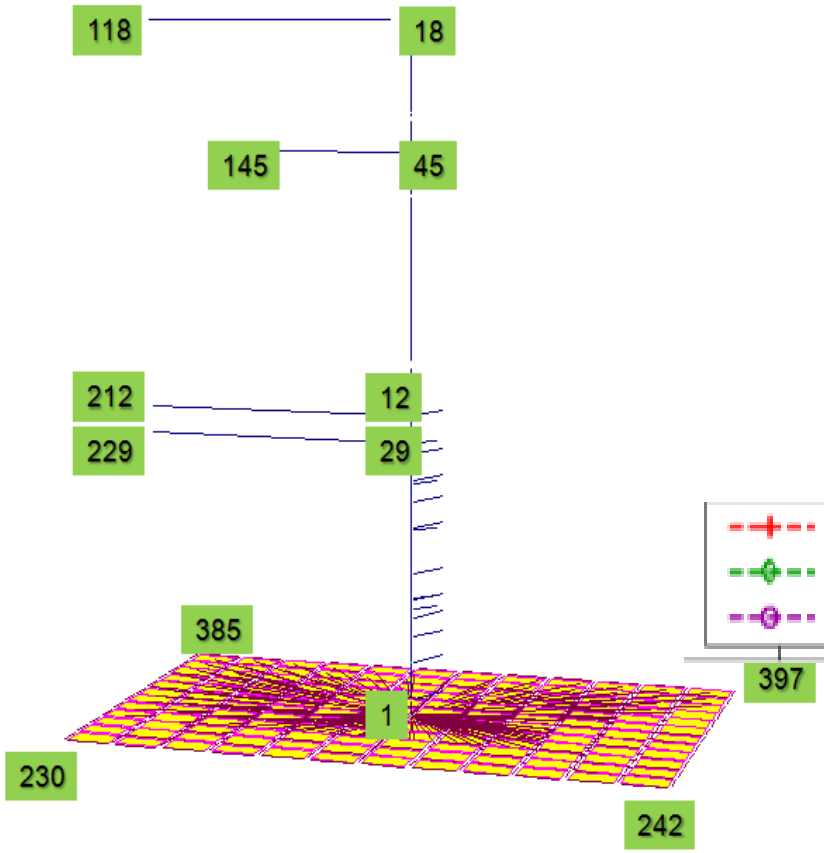
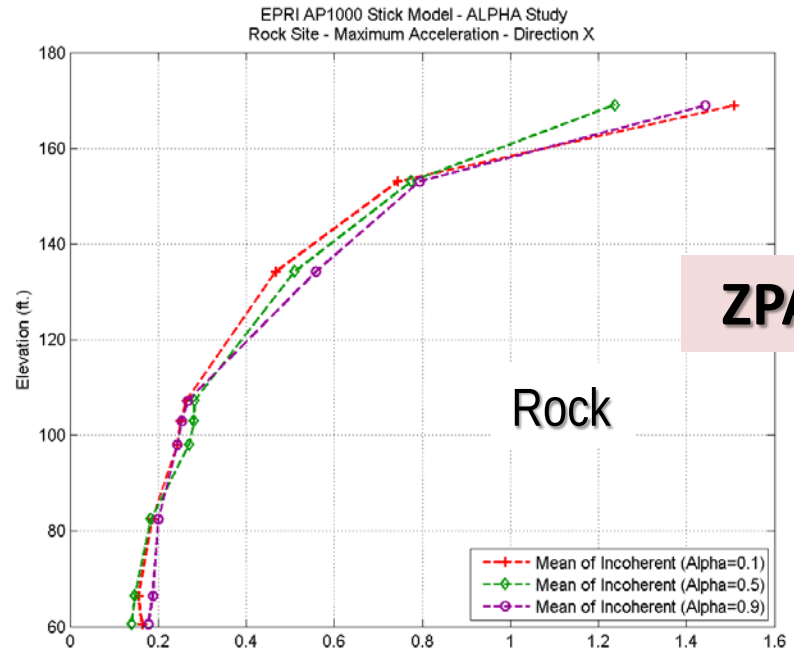
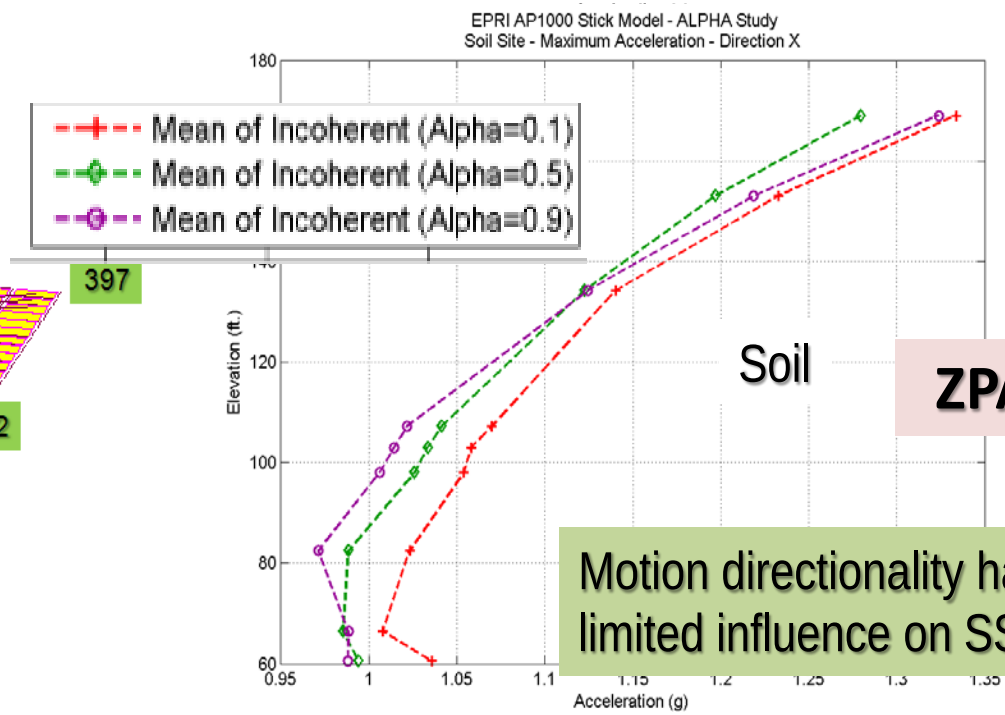
EPRI AP1000 NI Stick Model Study







- Soil Site, Vs = 1,000 fps, CSDRS, AB2007Soil
- Rock Site, Vs = 6,000 fps, EPRIRockRS, AB2007Rock

Motion directionality has limited influence on SSI

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Conclusions for Investigated Cases

- Incoherent motion describes a realistic, 3D random wave field motion.
- For realistic, elastic foundations, truncating the number of incoherent modes produces unconservative results in the high-frequency range.
- Zeroing the incoherent motion phasings produces overly conservative results in mid-frequency range at the price of the loss of physics – spatial correlation between support motions is neglected. Not applicable to the multiple time history analysis of RCL system.
- SSSI effects are significant for soil sites and possibly non-negligible for rock sites. Affect ISRS, soil pressures, foundation wall bending. Affect less the shear forces in the structure.
- Incoherent SSI effects are larger or different than the coherent SSSI effects.
- Incoherency model directionality, radial vs. directional, produces less significant effects on SSI response.

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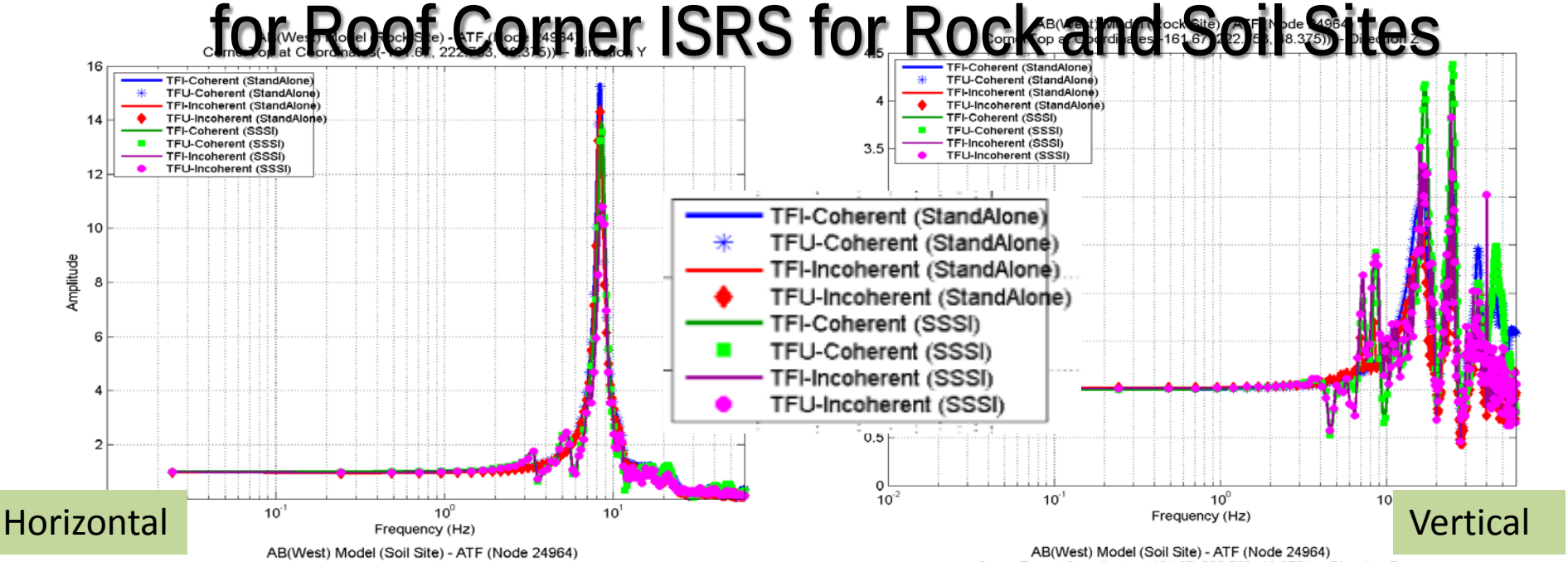
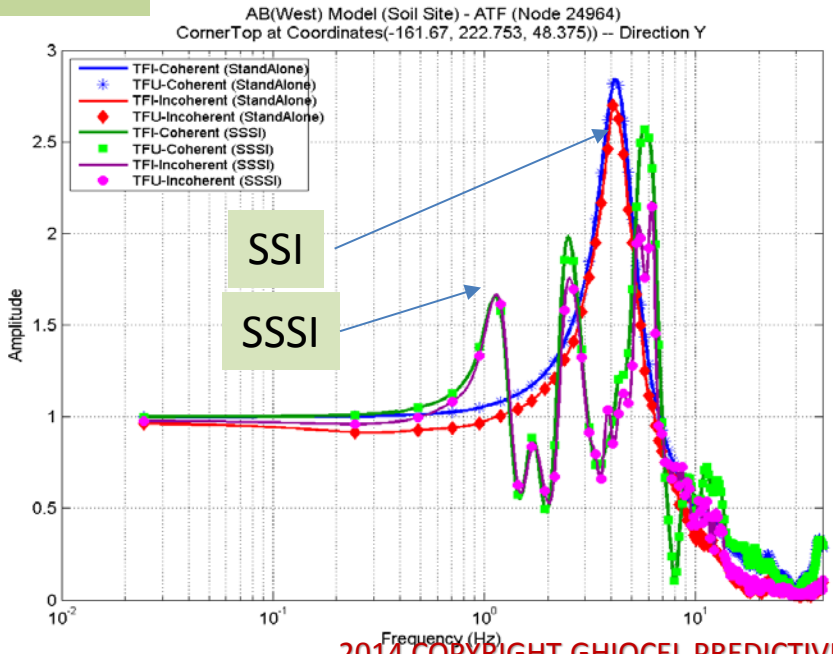
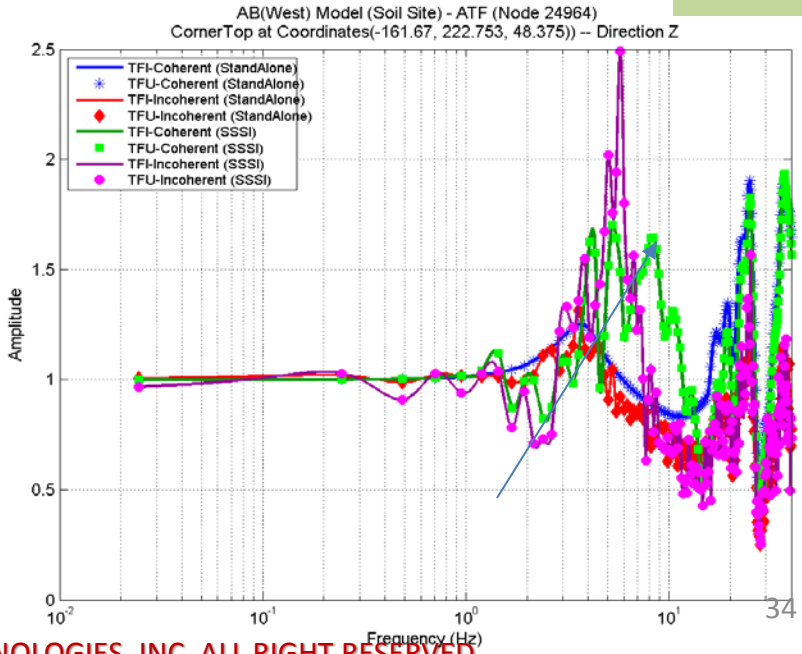
ABW Bldg. Coherent vs. Incoherent SSI and SSSI ATF for Roof Corner ISRS for Rock and Soil Sites







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